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REPORT on GEOTECHNICAL ASSESSMENT

PROPOSED NORTHERN EXTENSION OF GERROA SAND QUARRY GERROA AND BEACH ROADS, GERROA

Prepared for CLEARY BROS (BOMBO) PTY LTD

Project 37673 June 2006



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REPORT ON GEOTECHNICAL ASSESSMENT PROPOSED NORTHERN EXTENSION OF GERROA SAND QUARRY GERROA AND BEACH ROADS, GERROA

1. INTRODUCTION

This report presents a geotechnical assessment of the proposed Northern Extension of the Gerroa Sand Quarry at Gerroa and Beach Roads, Gerroa. The assessment was requested by Cleary Bros (Bombo) Pty Ltd, the operators of the quarry.

It is understood that Cleary Bros (Bombo) Pty Ltd are seeking approval from the Minister for Planning to extract the sand resource, extending some 800 - 900 m northeast of the existing dredge pond area, over a period of about 15 years.

The assessment comprised a review of published and unpublished data relevant to the quarry, a visit by a senior geotechnical engineer, cone penetration testing and boring with sampling, followed by chemical and physical testing of selected samples and geotechnical analysis of resource volume and geotechnical constraints to the development. The details of the field work and subsequent analysis are given below and include reference, where appropriate, to the previous assessments and data.

As required by the Department of Environment and Conservation (NSW) in the Director General's Requirements, the assessment has included consideration of relevant geotechnical and groundwater issues included in:

- Water Quality and River Flow Interim Objective 'Illawarra Catchment' (October 1999);
- Acid Sulfate Soil Manual, NSW Acid Sulfate Advisory Committee (August 1998);

- State Water Management Outcomes Plan;
- State Environmental Planning Policy No. 58 Protecting Sydney's Water Supply;
- Environmental and Health Protection Guidelines: Onsite Sewage Management of Single Households (February 1998);
- NSW EPA, Draft Environmental Guidelines for Industry: *The Utilisation of Treated Effluent by Irrigation* (February 1995);
- NSW Department of Housing, Managing Urban Stormwater: Soils and Construction (4th Edition, March 2004).

The preparation of the assessment has also included consideration of issues included in the Department of Primary Industries, Mineral Resources Division EIS Resource Data requirements.

The principal information sources comprised:

- 'Sand Resources in the Wollongong Area'. NSW Geological Survey Report No. GS1979/071 (Unpublished);
- 'Gerroa Sand Resource Southern Extraction Area'. R W Corkery & Co. Pty Ltd Report No. 279/02;
- 'Wollongong 1:250 000 Series Geological Series Sheet'. NSW Department of Mines, 1966;
- periodic rainfall, extraction face location, dredge pond level and, groundwater monitoring bore data collected by Cleary Bros (Bombo) Pty Ltd;
- materials testing carried out by Coffey Partners International Pty Ltd (Job No. SC568/1, July 1990);
- materials testing carried out by Network Geotechnics Pty Ltd (Job No. W2099/1, July 2000);
- the results of a Douglas Partners investigation (Project 25766, February 2001) of the Gerroa Sewage Treatment Works located approximately 1 km to the north of the proposed extraction area and situated within an equivalent depositional environment.

2. SITE DESCRIPTION

The site for assessment comprises an irregularly shaped area, generally ranging from 80 – 200 m wide, extending some 800 - 900 m in a north-eastern direction from the current northern extent of the operating dredge pond (Drawing 1). The site lies at the western side of Gerroa Road and is approximately 600 m from the current beachfront.

Natural surface levels relative to Australian Height Datum (AHD) range from RL 1, adjacent to a drainage canal near the northern end of the proposed extraction area, to RL 5 – RL 7 (AHD) along the Gerroa Road frontage.

Within the current dredge pond at the southern end of the proposed extraction area, sand extraction has been carried out to depths of up to about 4 m below dredge pond water level, corresponding to about 10 m below the level of Gerroa Road.

Limited clearing of vegetation has taken place progressively in front of the current extraction face beyond which the proposed extension area is densely tree covered for a length of about 200 m, thence partially tree covered to cleared over the remaining sections (Drawing 1).

3. GEOLOGICAL AND HYDROGEOLOGICAL SETTING

Reference to the Wollongong 1:250 000 Geological Series Sheet indicates that the Gerroa Sand Quarry and proposed Northern Extension lie within the drainage basin of Crooked River which discharges to the Shoalhaven Bight approximately 3.5 km to the northeast.

The basin is bounded to the north-west (at about the alignment of the South Coast Railway some 1.5 km to the northwest) by a topographic bedrock high of Berry Siltstone of Permian age. An east-trending spur of this bedrock high also extends to near the intersection of Gerroa and Beach Roads.

The bedrock is overlain by sediments of Quaternary (Holocene) age, which may be separated into the following broad deposition modes in order of surface occurrence from the present day beach:

- beach ridges located between the current seafront and the eastern side of Gerroa Road comprising aeolian sand. The beach ridge system controls the local creek drainage which flows northeast before joining the Crooked River.
- low, aeolian sand dunes extending 100 m to 500 m from the beach ridges.
- aeolian sand sheets extending 100 m to in excess of 1 km inland from the low dunes.
- fluvial and back dune lagoonal sediments comprising inter-banded sands, clay and mud. These deposits within Foys Swamp extend westerly from the edge of the sand sheet to the South Coast Railway.

The Gerroa Sand Quarry and the proposed Northern Extension Area are located at the rear of the beach ridge system on low sand dune and sand sheet deposits.

Cleary Bros (Bombo) Pty Ltd monitoring bores in the Gerroa Sand Quarry and the area extending north-east to adjacent to the Crooked River indicate that the alluvium acts as an aquifer in which there is a moderate variation in groundwater levels but a consistent, north-east trending flow gradient (about 0.3%) adjacent to the dredge pond, possibly reflecting the topographic bedrock high adjacent to southern side of Beach Road. Elsewhere, there appears to be a generally easterly-trending flow gradient of about 0.1% - 0.2% towards the shore. There are however appear to be locally even flatter gradients and reversals of gradient, suggesting that groundwater mounding within the dunes sheds both eastward to the sea and westward to the main drainage canal which continues northward as Blue Angle Creek and thence Crooked River (both of which are tidal).



4. FIELD WORK METHODS

The project field work comprised:

- a site inspection by a senior geotechnical engineer on 18 November 2004 for site familiarisation and selection of representative test locations.
- eleven cone penetration tests (CPT 101 111) to depths of 15 m within and adjacent to the proposed extension area. In these tests a 35 mm diameter instrumented cone is pushed using hydraulic thrust from a 15 tonne capacity truck-mounted unit with a vertical penetration rate of 20 mm/sec. Cone tip and sleeve friction resistance are monitored and recorded on computer for later plotting.
- four sampling bores (Bores 201 204) at locations selected on the basis of the results of the cone penetration testing. The bores were drilled to depths ranging from 13.5 – 17.5 m using a truck-mounted Scout auger/rotary rig, the bores being initially progressed using spiral flight augers until caving conditions were encountered, then extended to final depths using rotary methods. Representative samples were collected at 1 m depth intervals using auger and standard penetration test (SPT) methods.
- the sampled materials were transported and stored in accordance with standard protocols for subsequent testing of acid sulphate soils. The logging of the sampled materials was by an experienced engineering geologist.

Previous relevant materials investigations of the immediate site area have comprised:

- two sand sampling bores (SM 5 and SM 6), drilled to depths of 9 m by the Department of Mines in 1979. Sampling and logging was carried out over 1.5 m intervals.
- three sand sampling bores (CB 201, CB 204 and CB 206), drilled to depths of depths ranging from 5.5 14 m by Cleary Bros (Bombo) Pty Ltd in 1990. It is noted that the site of CB 201 has been mined and now lies within the area of the dredge pond.
- a test pit (TP 3) excavated to a depth of 7.5 m with sampling at approximately 1 2 m depth intervals by Cleary Bros (Bombo) Pty Ltd in 2000.

The locations of the current bores and cone penetration tests are shown on Drawing 1 and were determined using a hand held GPS unit. The collar levels were interpolated from contour data



included on an orthophotomap (an extract of which is included in Drawing 1) supplied for use in the assessment. The locations of the previous test locations are also shown on Drawing 1 and were scaled from drawings included in the previous reports for the site and as such, these locations are considered approximate only.

5. FIELD WORK RESULTS

Details of the conditions encountered in the current bores and the cone tests, together with previous subsurface investigations, are presented in Appendix A, together with notes defining classification methods and descriptive terms. Interpretative geological cross sections (Sections A - A' to G - G') summarising the results of the investigations are given in Drawings 2 and 3.

The test pit, bores and cone penetration tests intersected a regular pattern of soil and bedrock stratigraphy at the site generally consistent generally with the geological mapping.

Previous investigations summarised by R W Corkery & Co. Pty Ltd included subdivision of the sediments underlying the Gerroa Sand Quarry and part of the proposed Northern Extension Area into three units. The current detailed profiling by cone penetration testing and bores has permitted clearer definition of sediments and has permitted an extension of the previous unit descriptions as summarised below and shown on Drawings 2 and 3:

Unit 1: yellow to cream or orange brown, variably very loose to very dense, well sorted, fine to very fine grained sand and silty sand becoming fine to medium grained in places, with no shells, associated with dune deposits. The base of the unit is generally above mean sea level (about RL 0) but with isolated sections possibly extending down to RL -1. A 0.2 – 0.6 m thick organic rich silty sand topsoil was developed on the unit at Bores 201 – 204.

The unit varies in thickness from less than 1 m at the western edge of the proposed Northern Extension Area to approximately 7 m in Bore 203 adjacent to Gerroa Road.

Unit 2: fawn or orange-brown to grey, indurated in places, generally medium dense to very dense (with some loose bands), moderately well to poorly sorted, fine to very coarse grained sand with variable amounts of lithic gravel and shell, associated with a combination of beach, shore-face, barrier wash-over and tidal inlet deposits.

The base of the unit was interpreted at between RL -2 to RL -6, the unit thinning to the west and varies in thickness between about 4 m and 8 m. The unit was separated from the overlying Unit 1 by a clay lens (to about 0.3 m) in the area between CPT 104 and Bore 204.

Unit 3: dark grey to black, organic-rich, soft to stiff clay of estuarine or lagoonal deposition. The maximum thickness of the unit was approximately 2.5 m in the south-western section of the proposed Northern Extension Area. The unit thinned rapidly to less than 1 m or was absent (possibly being eroded during the marine transgression) in the north-eastern section of the area.

The base of the unit was interpreted at between RL -6 to RL -8. It is noted that a similar 0.5 - 1.5 m clay layer with a base at RL -3.5 to RL -4 was penetrated by testing at the Gerroa Sewage Works site.

The unit is interpreted as having formed in a back-dune location during a still-stand at about RL -5 preceding the stabilisation of sea level at about current levels some 6500 years before present.

Unit 4: grey to dark grey, generally medium and coarse grained, medium dense to very dense sand with included gravelly sand/sandy gravel bands, particularly at the base of the unit at the north-eastern end of the proposed Northern Extension Area.

The unit was up to 4m thick, base of the unit was interpreted at between RL -6 to RL -11. The unit generally thickened and deepened to the northeast.

Unit 5: grey, soft to very stiff and hard clay, interpreted as representing possible some alluvial material and the residual weathering profile developed on the Berry Siltstone. The maximum thickness penetrated by testing was 6 m. A similar profile was intersected at depths of 16 m to 18 m (RL –12 to RL –14) in the Gerroa Sewage Works site.

Groundwater was noted in the current bores and inferred at hole collapse depths at CPT locations at RL 0.5 - 2.5. In comparison, maximum groundwater levels of approximately RL 0.9, RL 1.4 and RL 1.6 are indicated by data obtained by Cleary Bros (Bombo) Pty Ltd in water monitoring bores WM 3, WM 4 and WM 5, respectively.



6. GROUNDWATER

The following sections assess hydrogeological factors determined from the available surface and groundwater monitoring data collected in the period 1993 – 2006. Where relevant, the available data has been compared to long-term rainfall records.

6.1 Rainfall and Pond Level

The site is located immediately adjacent to the Shoalhaven Bight and within an area characterised by a median and mean annual rainfall of about 1165 mm and 1252 mm, respectively (on the basis of over 100 years records at the Kiama and Point Perpendicular monitoring stations). Detailed (daily) rainfall records from the Gerroa Sand Quarry indicate a median and mean annual rainfall of about 1047 mm and 1065 mm, respectively, some 10% to 15% below the anticipated long-term median and average values for the local coastal area and some 6% to 9% greater than the Nowra monitoring station about 13 km inland.

Measurements of the dredge pond level have been compared to available yearly rainfall for the site in the period 1993 – January 2006 and the anticipated median rainfall value (Table 1).

	1993	1994	1995	1996	1997	1998	1999	2000	2005	2006
Maximum Level (AHD)	1.9	1.88	1.875	1.825	1.85	2.175	2.175	1.65	1.65	1.1
Minimum Level (AHD)	1.4	1.27	1.0	1.35	1.35	0.95	1.15	0.95	1.15	1.05
Range of Level (m)	0.5	0.53	0.875	0.475	0.5	1.225	1.025	0.7	0.5	0.05
Total Rainfall (mm)	755	926	1134	866	1101	1440	1309	993	1147	NA
Variation from Median Rainfall (mm)	-410	-239	-31	-299	-64	+275	+144	-172	-18	NA

 Table 1 – Summary of Yearly Rainfall and Dredge Pond Levels

NA = Not available



The results shown in Table 1 indicate that:

- 1998 and 1999 had rainfall in excess of the anticipated median (1165 mm) rainfall.
- the yearly maximum dredge pond level in years of less than median rainfall moved within a limited range (RL 1.65 – 1.9) with an average maximum of RL 1.83.
- the increase in dredge pond level in 1998 and 1999 corresponded closely with the rainfall in excess of the median value.
- the yearly minimum dredge pond level also moved within a limited range (RL 0.95 1.4).
- the minimum dredge pond level (RL 0.95) is 0.45 m above mean sea level.
- the maximum dredge pond level (RL 2.175) occurred during the year of highest rainfall (1998) indicating the rapid effect of rainfall on the groundwater regime.

The dredge pond levels have also been compared to daily rainfall events and the results of the 1999 - 2000 record periods (when the dredge pond area was slightly smaller than the current case). The comparison indicates that:

- for daily rainfall events generally in excess of 100 mm or close spaced rainfall events totalling about 100 mm there is a similar rise in the dredge pond level.
- high dredge pond levels declined rapidly towards the minimum (base) level between August 1999 (an above average rainfall period) and June 2000 (within a below average rainfall period) with the decline being stabilised in the February to May 2000 period by some seven rainfall events in the range 10 mm to 50 mm.

When dredge pond levels are compared to the levels in the groundwater monitoring bores, there is an indication that, during years of significantly lower than median rainfall (e.g. 1993), there is a possible localised reversal of groundwater flow at the eastern end of dredge pond (see Drawing 4) compared to a normal or slightly below median rainfall period (e.g. 2000) when flow gradient continues to the northeast (see Drawing 5).



6.2 Rainfall and Evaporation from Dredge Pond

The assessed median rainfall for the Gerroa area is about 1165 mm (in comparison with 1047 mm for the period of recording at the Gerroa Sand Quarry). In comparison, available data for evaporation rates for a NSW south coast area recording station (Ulladulla) indicates that the average daily evaporation rate is of the order of 2.8 mm/day (1022 mm/year), less than either the assessed or measured rainfall data. It is however understood that readings at the Nowra monitoring station indicate average evaporation of the order of 4 mm/day (about 1460 mm/year). As such, there is an uncertainty with regard to the balance between rainfall and evaporation from the dredge pond at the site.

6.3 pH of Dredge Pond Water and Groundwater

Measurement of the pH of both the dredge pond water and groundwater in the Cleary Bros (Bombo) Pty Ltd monitoring bores has been carried out on a regular basis since 1993. The monitoring of the dredge pond and the monitoring bores WM 1 and WM 2 at the southern end of the Gerroa Sand Quarry, together with monitoring bores WM 3, WM 4, WM 5 and WM 6, the main drainage canal and Blue Angle Creek in or near the proposed Northern Extension Area indicated:

- the dredge pond pH has generally moved within the range 6.0 9.0 (moderately acidic to strongly alkaline) in comparison with a range of 5.4 – 8.5 (strongly acidic to strongly alkaline) for the monitoring bores.
- the lowest dredge pond pH values were measured in the period of heavy rainfall at the end of July 1998 and extreme rainfall in mid August 1998. This may reflect the flushing of organic acids or oxidised pyritic material from the sand aquifer.
- the minimum pH levels (pH 5.4 and 5.9, respectively) in the monitoring bores WM 2 and WM 3 may be an indicator of pyrite oxidation or the presence of organic acid complexes.
- the pH of Blue Angle Creek at the flood gates at the northern end of the CB property (i.e. north of the proposed quarry extension) generally ranged between 6.6 and 7.8, but with a lower pH reading of 4.8 being associated with transient stream flushing event during wet



weather. For comparison, pH readings as low as 3.2 have been recorded in the drains within Foys Swamp, upstream (west) of the proposed quarry extension area.

The conditions represented by the pH values within the dredge pond or monitoring bores described above are not considered severe. The significantly lower pH values recorded in the Foys Swamp drainage system and its discharge path (Blue Angle Creek) indicate periodic severe conditions probably related to flushing of acid formed by oxidation of acid sulphate soils.

6.4 Groundwater Quality

It is noted that the Berry Siltstone, which forms the bedrock to the immediate south of Berry Beach Road, frequently includes accessory pyrite mineralisation, but of a significantly different formation type and form than associated with acid sulphate soils. The soils developed on the Berry Siltstone are commonly acidic and surface runoff from these soils may influence the quality of the groundwater within the adjacent alluvial aquifer.

Measurement of the electrical conductivity (as an indicator of Total Dissolved Solids [TDS]) of groundwater in the dredge pond and the Cleary Bros (Bombo) Pty Ltd monitoring bores WM 1 - WM 6 has been carried out since 1993. Additional field measurement of TDS has also been undertaken by E2W and others in the dredge pond and at Blue Angle Creek. The results of the monitoring are summarised in Table 2 (see following page).

It is understood that the highest value at Blue Angle Creek was recorded at high tide and indicates substantial mixing with seawater.

Field measurements of Dissolved Oxygen (DO) have also been undertaken recently by E2W and others in the dredge pond, main canal and at Blue Angle Creek. The results indicate DO values in the ranges 65% - 100%, 24% – 100% and 26% – 92% in the dredge pond, main canal and Blue Angle Creek, respectively.





Period	Electrical Conductivity (µS/cm) at sampling location							
	WM 1	WM 2	WM 3	WM 4	WM 5	WM 6	Dredge Pond	Blue Angle Creek
1993	636 - 5980	416 - 920	130 - 470	404 - 605	96 - 1140	91 - 225	400 - 626	NA
1994	390 - 2940	395 - 1162	132 - 1833	142 - 930	86 - 1140	495 - 966	NA	NA
1995	650 - 2440	322 - 1983	140 - 380	388 - 1620	96 - 490	76 - 209	NA	NA
1996	414 - 4620	321 - 1114	213 - 609	376 - 590	98 - 120	91 - 136	NA	NA
1997	2390 - 4980	335 - 501	214 - 537	351 - 677	109 - 180	73 - 120	NA	NA
1998	1220 - 6470	366 - 902	191 - 701	370 - 1052	54 - 171	93 - 236	NA	NA
1999	965 - 2900	455 - 879	161 - 314	264 - 1098	84 - 191	83 - 238	NA	NA
2000	739 - 3830	460 - 770	NA	604 - 1865	NA	113	NA	NA
September 2003-2004	278 - 1113	465 - 530	NA	NA	NA	NA	510 - 1339	750 – >20000
2005	290 - 1030	NA	NA	740 - 1110	100	NA	510 - 690	810 - >20000
Range over measurement period	278 - 6470	321 - 1983	130 - 1833	142 - 1865	54 - 1140	73 - 238	400 - 1339	750 - >20000

Table 2 – Summary of Electrical Conductivity (µS/cm) at Sampling Locations

NA Not available

The results shown in Table 2 indicate that:

- monitoring bore WM 1, located up-gradient and closest to bedrock exposure, has the consistently highest yearly TDS values (where TDS ≈ 0.65 x µS/cm), consistent with flushing of salts from the bedrock.
- the highest minimum yearly TDS values in the monitoring bores were recorded in the upgradient bores WM 1 and MW 2, consistent with flushing of salts from the bedrock.
- the increased minimum TDS values for the dredge pond in comparison with the surrounding groundwater mass are likely to relate to the disturbance caused by the dredging and processing procedures.
- all minimum conductivity values indicate fresh water conditions (<1500 μS/cm of the Clean Waters Regulations).



- the maximum conductivity values for the dredge pond and monitoring bores WM 5 and WM 6 indicate fresh water conditions.
- monitoring bore MW 5 has the overall highest quality water, possibly related to mounding of infiltrated rainwater within the dune sand at that location.
- the surface water quality in Blue Angle Creek fluctuates with tide, being fresh at low tide and saline at high tide.

7. LABORATORY TESTING

7.1 Physical Testing

Selected samples from the current bores were tested in the laboratory for measurement of particle size distribution (grading), particle density, water absorption and organic impurities (other than sugar). The detailed results of the current testing are given in Appendix B and additional test results of particle size distribution, shell (carbonate) content and organic impurities from the previous investigations are given in Appendix C.

The results of all testing are summarised below.

7.1.1 Grading

A total of 29 particle size distributions were determined on samples from within or adjacent to the proposed extension area. The results are summarised in Table 1 (following page) with comparison with the grading requirements and limits of deviation for uncrushed fine aggregate for use in concrete (Australian Standard AS 2758.1 – 1998).

The results indicate that the very fine and fine grained sands of Unit 1 generally do not satisfy the requirements of AS 2758.1 – 1998, the samples generally exceeding the requirement for the passing 300 μ m size range. The unprocessed, medium and coarse grained sands obtained from Unit 2 and Unit 5 generally satisfy the requirements with minor exception of small excesses in the coarser and 75 μ m size ranges.

It is understood that the mixing of Unit 1 and Unit 2 materials during dredging and subsequent processing have provided acceptable fine concrete aggregate during the operation of the Gerroa Sand Quarry.

Location	Depth	Unit	Percentage Passing (%)							
	(m)		9.5	4.75	2.36	1.18	600	300	150	75
			(mm)	(mm)	(mm)	(mm)	(µm)	(µm)	(µm)	(µm)
Bore 201	1.0	1	100	100	100	100	100	98	12	2
	2.0	1	100	100	100	100	100	92	5	1
	8.0	2	99	99	98	92	78	49	13	4
	11.0	4	100	100	100	99	89	41	8	3
Bore 202	1.5	1	100	100	100	99	98	98	9	1
	3.0	1	100	100	100	100	100	90	6	2
	5.0	2	100	100	100	99	98	78	7	2
	7.0	2	99	98	97	92	80	46	7	4
	10.0	4	98	97	96	94	73	27	10	6
Bore 203	2.0	1	100	100	100	100	100	94	11	2
	6.0	1	100	100	100	100	100	83	7	2
	9.0	2	100	98	94	85	71	39	10	3
	12.0	4	100	100	99	98	89	34	7	3
Bore 204	0.5	1	100	100	100	100	100	99	9	2
	2.0	1	100	100	100	100	100	86	5	2
	4.0	1	100	100	100	100	100	78	3	1
	7.0	2	98	97	94	90	86	47	8	3
	12.0	4	100	100	99	94	59	13	4	2
CB201	3.0 – 5.0	2	100	98	97	95	77	38	12	6
	4.9 – 5.4	2	95	94	92	88	76	43	13	5
CB204	0 – 1.0	1 - 2	100	100	100	100	98	86	17	12
	2.0 - 5.0	2	100	98	97	91	60	19	6	4
CB206	6.0 - 8.0	2	100	96	92	90	85	63	19	10
	8.0 - 10.0	4	100	97	94	88	73	44	10	6
	10.0 -13.0	4	100	100	100	99	84	41	8	3
TP3*	1.2	2	100	100	100	99	80	22	10	8
	4.0	2	100	100	100	99	89	40	13	7
	6.0	2	100	100	100	99	76	12	5	2
	7.5	2	100	100	100	100	89	33	10	5
AS 2758.1 – 1998										
LIMITS			100	90 -	60 -	30 –	15 –	5 – 50	0 - 20	0-5
(MASS PASSING %)				100	100	100	100			
MAXIMU	M DEVIATIO	N (%)	-	±5	±5	±10	±15	±10	±5	-
Note		*	Washed	Samples	3					
			Outside	Limits						

Table 1 – Summary of Particle Size Distribution Testing – Proposed Northern Extension Area

7.1.2 Particle Density, Water Absorption and Organic Impurities

Five samples were tested for particle density and water absorption. An additional three samples were tested for organic impurities (other than sugar). A further four results for organic impurities were available from the 2000 testing of samples from TP 3.



The particle density results (on dry basis) ranged from 2.33 - 2.37 t/m³, satisfying AS 2758.1 – 1998 requirements (greater than 2.1 t/m³) for normal weight aggregate. Water absorption results ranged from 0.3 - 1%, satisfying the AS 2758.1 – 1998 requirement (about 2%).

Organic impurities testing of current samples and samples from TP3 indicated three 'pass' and four 'fail' results, indicating a need for processing to reduce the organic content.

Shell content (by a non-standard method) was previously determined on samples selected from Bores CB 201, CB 204 and CB 206. Carbonate content by acid digestion was also carried out on washed samples from TP3. The results of these tests are given in Table 2.

Location	Depth	Unit	Shell >1.18mm (%)	Shell <1.18mm(%)	Total Carbonate (%)
CB201	3.0 – 5.0	2	-	8.2	
	4.9 – 5.4	2	10.2	6.2	
CB204	0 – 1.0	1 - 2	-	7.5	
	2.0 - 5.0	2	1.0	2.6	
CB206	6.0 - 8.0	2	34.9	20.3	
	8.0 – 10.0	4	12.0	6.3	
	10.0 -13.0	4	1.0	1.6	
TP3	1.2	2			<0.2
	4.0	2			<0.2
	6.0	2			<0.2
	7.5	2			<0.2

 Table 2 – Summary of Carbonate Content Testing

7.2 Chemical Testing

As part of the current investigation, chemical testing was carried out for assessment aggressivity and of acid sulphate soil potential. The detailed results are given in Appendix D and are summarised below.

7.2.1 Aggressivity

Three samples were tested for pH, chloride and sulphate content in 1:5 soil:water extracts. The results indicated pH values in the range 6.2 - 6.7, chloride in the range 1.5 - 52 mg/kg and sulphate in the range <2.0 - 8.1 mg/kg, these being generally indicative of non-aggressive or mildly aggressive ground conditions in accordance with Australian Standard AS 2159 – 1995.



7.2.2 Acid Sulphate Testing

Sixty nine samples collected from the test bores were screened by measurement of pH after the addition of distilled water (pH_F) and peroxide (pH_{FOX}). These screening tests give an approximate indication of either the presence of actual acid sulphate soils (AASS) or potential acid sulphate soils (PASS). On the basis of the screening tests, five samples were submitted to the SGS Environmental Services Laboratory for Peroxide Oxidation Combined Acidity and Sulphate (sPOCAS Method) testing.

Additional details of ASS potential are also available from the CB monitoring of Total Oxidisable Sulphur as part of the quality control of processed sand. The results of all acid sulphate soil testing are given in Table 3 (see following pages) are summarised below.

The screening and sPOCAS results indicated that:

- the pH_F levels of the samples were not indicators of AASS conditions.
- 4 samples had moderate or vigorous reaction on oxidation being positive indicators of potential acid sulphate soils (PASS) conditions.
- a total of 27 samples had significantly lower pH after oxidation, these also being positive indicators of PASS conditions. However, only one sample (Bore 201/13.0 m) gave a strongly indicative result (pH<3). The positive indicators were mostly obtained from bands within Unit 1, Unit 4 and Unit 5 materials.
- all sPOCAS samples had S_{pos} values at or in excess of the Action Criteria value (0.03%) for sandy materials as presented in Acid Sulphate Soils Assessment Guideline. The S_{TPA} values showed a significant buffering effect, probably due to shell fragment content. For risk assessment, however, the higher of the sulphur and acid "trails" should be used.
- the five processed sand samples from the Gerroa Sand Quarry analysed during the period October 2003 to December 2004 had Total Oxidisable Sulphur values in the range 0.009% to 0.029%, less than or equivalent to the Action Criteria value (0.03%) when quoted to two significant places.



Location	Depth	Unit	Field Screening Tests			sPOCA	S Test	
	(m)		Natural	Oxidised	pH _F .	Effervescence	S _{pos} %	S _{TPA} %
	. ,		pH _F	pH _{FOX}	pH _{FOX}		pee	
Bore 201	0.5	1	6.2	5.9	0.3	S		
	1.0	1	6.4	6.2	0.2	S		
	1.5	1	6.5	6.2	0.3	S		
	2.0	1	6.6	5.8	0.8	S		
	3.0	1	6.7	6.3	0.4	S		
	4.0	1	7.0	6.4	0.6	S		
	5.0	2	7.9	7.7	0.2	S	0.03	<0.01
	6.0	2	7.8	7.9	-0.1	S		
	7.0	2	7.9	8.2	-0.3	S		
	8.0	2	8.2	7.7	0.5	S		
	9.0	2	8.4	7.4	1.0	S		
	10.0	4	8.1	5.9	2.2	S		
	11.0	4	8.1	5.6	2.5	S		
	12.0	4	8.0	4.7	3.3	S		
	13.0	4	7.9	2.5	5.4	M	0.32	<0.01
Bore 202	0.5	1	8.3	4.8	3.5	S	0.08	< 0.01
	1.0	1	8.3	5.8	2.5	S		
	1.0	1	7.9	5.6	2.3	S		
	1.5	1	8.0	5.8	2.2	S		
	2.0	1	6.8	7.0	-0.2	S		
	3.0	1	6.9	6.8	0.1	S		
	4.0	2	7.4	6.8	0.6	S		
	5.0	2	8.2	7.4	0.8	S		
	6.0	2	7.9	7.4	0.5	S		
	7.0	2	7.7	7.6	0.1	S		
	9.0	2	7.7	7.7	0.0	S		
	10.0	4	7.7	5.6	2.1	S		
	11.0	4	7.7	4.8	2.9	S - M		
	12.0	4	7.7	5.9	1.8	S		
	13.0	4	6.6	6.2	0.4	S		
	14.0	4	6.8	6.2	0.6	S		
	15.0	4	7.0	6.2	0.8	S		
Bore 203	0.5	1	7.5	6.0	0.5	S		
	1.0	1	7.2	6.1	1.1	S		
	1.5	1	7.3	6.0	1.3	S		
	2.0	1	7.3	6.4	0.9	S		
	3.0	1	7.7	6.5	1.2	S		
	4.0	1	7.4	6.5	0.9	S		
	5.0	1	7.4	6.5	0.9	S	0.09	<0.01
	6.0	1	7.4	6.8	0.6	S		
	7.0	1	7.9	7.4	0.5	S		
	8.0	2	7.7	7.5	0.2	S		
	9.0	2	7.5	6.8	0.7	S		
	10.0	2	7.4	7.1	0.3	S		
	11.0	2	7.6	7.5	0.1	S		
	12.0	4	7.9	7.2	0.7	S		
	13.0	4	7.9	7.2	0.7	S		
	14.0	4	7.9	7.0	0.9	S		
	15.0	4	7.9	6.9	1.0	S		

Table 3 – Summary of Screening and Analytical Results



Location	Dept	Unit		Field Scr	sPOCA	S Test		
	h		Natural	Oxidised	pH _F .	Effervescence	S _{pos} %	S _{TPA} %
	(m)		рН _F	pH _{FOX}	pH _{FOX}		•	
Bore 204	1	0.5	7.3	6.2	1.1	S		
	1	1.0	7.4	6.2	1.2	S		
	1	1.5	7.4	6.5	0.9	S		
	1	2.0	7.4	6.2	1.2	S		
	1	2.5	7.4	6.1	1.3	S		
	1	3.0	7.3	6.1	1.2	S		
	1	4.0	7.2	6.1	1.1	S		
	1	5.0	7.9	7.9	0.0	S		
	2	6.0	7.9	7.1	0.8	S		
	2	7.0	8.2	7.3	0.9	S		
	2	8.0	8.2	7.7	0.5	S		
	2	10.0	6.3	6.4	-0.1	S		
	4	11.0	6.2	6.4	-0.2	S		
	4	12.0	6.5	6.4	0.1	S		
	4	13.0	6.6	6.5	0.1	S		
	4	14.0	7.7	6.5	1.2	М	0.89	0.47
	5	15.0	7.8	6.5	1.3	V		
Processed	Sand St	ockpiles						
GS1							0.029	
(13/10.03)								
GS2							0.025	
(13/10/03)								
GS1							0.022	
(17/08/04)								
GS2							0.028	
(17/08/04)								
GS							0.009	
(13/12/04)								

Note: Bold indicates positive indicator S = Slight M = Moderate V = Vigorous

8. COMMENTS

8.1 **Proposed Development**

Cleary Bros (Bombo) Pty Ltd are seeking approval from the Minister for Planning to extract the sand resource from the Northern Extension Area, which extends some 800 - 900 m northeast of the existing dredge pond area, over a period of about 15 years. The approximate outline of the area is given on Drawing 1.



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It is expected that an excavation face ranging from 80 m to 160 m wide will be progressively moved northward from the current dredge pond and that excavation depths of up to 17 m will potentially be developed to recover materials from Units 1, 2 and 4 within the area shown on Drawings 1 and 2. The closest approach of the extraction area to the main canal will be 40 m and there will be a buffer, some 25 m wide, will be provided for screening bunds between the extraction area and road reserve (Gerroa Road). It is understood that the existing processing area at the western edge of the current dredge pond will also be used for the proposed extraction operations.

8.2 Geological Model of Resource

The geological model for Northern Extension Area resource, as summarised in Drawings 2 and 3, comprises an upper, very fine to fine grained dune sand (Unit 1) underlain by generally medium to coarse grained sands of beach and tidal inlet deposits (Units 2 and 4). Clayey materials (Unit 3 and possibly the upper section of Unit 5) of lagoonal or back swamp depositional mode, which are likely to include sulphidic materials, form semi-continuous lenses to 3 m thick within the south-western section of the area, but are discontinuous and generally less than 1 m thick in the remaining sections. The area is characterised by a groundwater table that ranges between a base level of RL 0.95 and RL 2.2 in response to rainfall.

The resource is partially affected by the presence of potential acid soils, mostly within the deeper sections of Unit 4 which may include pyritic materials eroded from the underlying Unit 5 during the marine transgression leading to the current sea level. The positive indicators PASS within Unit 1 are considered to be anomalous to the aeolian deposition mode and may result from clayey particles blown from the Foys Swamp area, which is recorded on acid sulphate risk maps as being of high probability of acid sulphate soil conditions.

Materials from Units 1, 2 and 4 form the recoverable resource. The processing of the very fine to fine grained sands of Unit 1 with the underlying Units 2 and 4 sands, which extend to depths of 17 m, is expected (on the basis of the satisfactory performance of the Gerroa Sand Quarry) to produce fine concrete aggregate and reduce pyritic materials to acceptable levels by appropriate processing.



8.3 Acid Sulphate Soil Risk

The previous sand extraction within Units 1 and 2 has been satisfactorily managed the risk of acid generation and heavy metal release associated with the acid sulphate soils and the continued extraction of these units should also be expected to result in a satisfactory outcome. However, as a consequence of the exceedance of the *Action Criteria* in some Unit 1 materials (although considered to be anomalous results) and in some Unit 2 and Unit 4 samples, together with the significant volume of the proposed excavation, a detailed Acid Sulphate Soil Management Plan (ASSMP) is required. Planning and management options should therefore assume that, unless otherwise indicated by site-specific testing before or during excavation, all materials of estuarine origin (Units 2, 3 and 4) and the site in general need to be tested and/or monitored. The excavated Unit 4 materials of acid sulphate soil risk may require specific processing such as sluicing or hydrocycloning, the extent of which will need to be determined during the on-going extraction operation.

It is considered that an appropriate ASSMP should include:

- continuation of the current surface, groundwater and dredge pond water quality monitoring prior to, during and subsequent to the extraction process.
- installation of additional monitoring bores in the buffers between of the proposed extension area and the main canal and Gerroa Road, together with an additional monitoring bore to the north of the proposed extension area.
- additional testing of the acid sulphate soil potential to supplement the results of this investigation. This testing should be progressively carried out to permit selection of the final extraction areas and relevant treatment methods for the individual sections and/or units to be extracted within the resource.
- on-going monitoring of the feed stock and processed materials including reject slimes during the extraction.
- controlled placement of reject materials, including sulphidic fines and the oversize shell component from the processing (to assist in pH buffering) within the basal section of the dredge pond. The burial of these materials with non-sulphidic material may be appropriate.

• the holding on site of suitable quantities of buffering materials for addition to the dredge pond if modification of the pH is required on the basis of the on-going testing.

8.4 Plant Operation and Excavation Stability

The sand resource includes two distinct excavation environments; up to 4 m of very loose to very dense, very fine grained and fine grained sand and silty sand (Unit 1) lying above the water table and up to 13 m of fine grained sand (remainder of Unit 1) and medium dense to very dense, medium to coarse grained sand (Units 2 and 4). In general, an average lower-bound friction angle of about 30° is considered appropriate to the intersected sand profile.

The excavation of the profile above the water table would probably be stripped of topsoil and root affected sand (totalling an average of about 0.5 m in the current bores) by dozer operation, with the subsequent winning of materials by an excavator loading into trucks. Once the groundwater level is approached, it is anticipated that access difficulties would result in the use of the dredge system currently in use within the Gerroa Sand Quarry. Where the removal of the clays of Unit 3 to provide access to the underlying sand (Unit 4) is economic, it is probable that the soft to stiff clay would require the use of a cutter-suction type dredge, possibly with the assistance of a long-reach excavator mounted on a barge or working from the head of the excavation.

Observation of the working method within the Gerroa Sand Quarry and review of survey sections through the dredge pond area indicates that:

- water removed from the pond during dredging is returned almost directly to the pond via run-off from the discharge/processing area or via rapid infiltration of the sand profile about the working area.
- the working method does not lead to the extraction and disposal of the groundwater from the site. Rather, the pond water is recycled rapidly during the sand extraction process with possible minor additional evaporation. The records of the dredge pond pH indicates that while pyritic material is present within the sand resource, the exposure time during extraction, processing and stockpiling, is insufficient to cause complete oxidation and increase in the water acidity in comparison with the pH of the groundwater sampled from the



nearby monitoring bores. Alternatively, as suggested by the current testing, relatively benign pH could signify generally low pyrite contents and a buffering of the system by included shells.

The proposed extraction of the sand resource will need to considered the long-term stability of the dredge pond, such that there is no migration of the batters of the completed pond outside of the nominated resource and to this end, it is suggested that an average excavation slope of not greater than 25° (about 2.1H:1V) be employed.

It is anticipated that the stripped organics affected topsoil or silty sand and reject (slimes and larger shell fragments) materials will be placed into the completed dredged area. Suggested design criteria for the restored dredge pond (considered to be equivalent of a sheltered basin structure) are:

- a 6H:1V batter for required beach zones in accordance with current development conditions.
- a 2H:1V to 3H:1V maximum batter where re-vegetation and maintenance is required above the beach zone.
- an underwater maximum batter of 4H:1V at depths greater than 1 m below extreme low water level.

If sulphidic slimes won from processing are to be deposited in the base of the completed dredge pond, consideration will need to be given to any requirement for capping of these materials to promote or maintain an anaerobic deposition environment.

8.5 Groundwater Issues

The current EPA Licence and Development Consent require:

- a monitoring of discharged water at the overflow pipe from the dredge pond.
- monitoring of groundwater levels and water quality in the monitoring bores in and around the quarry and in the dredge pond, monthly and following any periods of extreme wet weather.

- water quality testing including, as a minimum, conductivity (a measure of total dissolved solids) plus pH and in the event that acid sulphate material is detected the possible requirement for monitoring of additional water quality parameters.
- flood bunding to RL 3.2.

Crooked River system, including Blue Angle Creek, is an uncontrolled lowland river for which relevant River Flow Objectives (RFO) and Water Quality Objectives (WQO) are:

- total phosphorus <25 μg/L
- total nitrogen <350 μg/L
- turbidity 6 50 NTU
- salinity 125 2200 μS/cm
- dissolved oxygen 85 110% saturation
- pH 6.5 8.5.

Applicable target water criteria (after ANZECC 2000 or NSW Clean Waters Regulations 1972 where no ANZECC Guidelines are available) are for surface discharge or for potential subsurface migration of water from the existing or proposed dredge pond to the groundwater or the adjacent "fresh" water drainage canal system.

- pH between 6.5 and 9.0
- Dissolved oxygen (DO) > 6 mg/L (>80 90% saturation)
- Total dissolved solids (TDS) <1500 mg/L
- Total suspended solids (TSS) <50 mg/L
- Fe (total) <0.5 mg/L and AI (total) <0.055 mg/L for pH >6.5.

The available chemical test results for surface and groundwater (see Appendix E) for the period 2004 and 2005 indicates that the water within the current dredge pond satisfied all ANZECC criteria with the exception of the Fe (total) value (which is expected to be naturally elevated in the geological environment including acid sulphate materials and weathering of pyritic iron which forms an accessory mineral of the underlying bedrock of the Berry Formation). The



dredge pond water is generally of higher quality than the up-gradient groundwater and surface water in adjacent waterways that pass through the back dune AASS and PASS deposits of Foys Swamp and discharge to Blue Angle Creek. During the period, the dredge pond water also met the WQO criteria for total phosphorus.

The monitoring results of the dredge pond since 1993 indicate that the pH of stored water was less than the NSW Clean Waters Regulation value of 6.5 only during the wettest year (1998). The lowest value (pH 6.0), which is within the ANZECC 2000 range for surface water, is assessed as resulting from flushing of acids generated by the oxidation of ASS. All electrical conductivity values for the dredge pond are within the ANZECC 2000 and WQO guidelines ranges.

The available chemical test results indicates that the extraction and processing of the sand from the proposed Northern Extension Area should not result in the pH and TDS guideline values given above being exceeded. As discharge of surface water (of higher quality than the receiving stream) from the bunded, inwardly drained site is not expected (no use of the discharge channel to date) and the groundwater gradient between the proposed dredge pond and the main drainage canal will be slight, there is minimal risk of the proposed development adversely affecting the quality of water in Blue Angle Creek.

The minimum groundwater level (RL 0.95) observed in the dredge pond to date represents a base level related to the tidal water levels in the adjacent tidal section of Blue Angle Creek and the main drainage canal. Consequently, the extraction of the sand from the Northern Extension Area should not result in drawdown of the groundwater table below the previously observed minimum level.

8.6 Resource Estimate

The following estimates (Table 4) of sand resource within the limits of the proposed Northern Extension Area as shown on Drawing 1 have been determined on the basis of:

• the stratigraphic profile interpreted from the current and past investigation data (Drawings 2 and 3).



- cross-sectional areas through the various units taken at 100 m intervals along the length of the proposed extraction area with the volumes being determined by average end-area methods.
- an excavation batter of 2.1H:1V both above and below the dredge pond level.
- a stripping depth of 0.5 m.
- limiting the suction dredging of the resource to above Unit 3 where this is present and greater than 1 m thick (about mid-distance between Sections F – F' and G – G' (Drawing 3).
- alternatively, removing the clay Unit 3 layer to a maximum of 2 m thickness.
- the assumption that all acid sulphate materials can be successfully processed for the intended final use and that environmental constraints can be managed.

Table 4 – Summary of Resource Estimate

Excavation Alternative	Volume of Sand Insitu (m³)	Volume of Unit 3 Removed (m ³)
Unit 3 Excavated <1 m thick	661 400	3 500
Unit 3 Excavated <2 m thick	677 500	25 700

9. SUMMARY

From consideration of the available data, site records and inspection and investigation, it is our opinion that:

- the proposed Northern Extension Area includes an estimated 661 400m³ of sand expected to be recoverable using a combination of the suction dredge (as current) and long-reach excavator operations in areas where included clay bands/lenses (Unit 3) are less than 1 m thick.
- the recovery of an additional 16 100 m³ of sand from the lowest sand unit (Unit 4) within the southern third of the area would require the removal of an additional (approximately) 22 200 m³ of clay (Unit 3) where the unit is between 1 m and 2 m thick.

- the proposed extraction within the Northern Extension Area sand resource will not result in variation in the range of groundwater levels or pH that have been previously experienced in the current dredge pond or adjacent water monitoring bores.
- appropriate engineering design can provide long-term stability of the dredge pond adjacent to adjacent forested areas and the easement of Gerroa Road.
- the final water body will not affect the overall commercial use of the aquifer. Average direct
 rainfall to the pond may be excess of the estimated yearly evaporation from the pond
 surface, the pH range (moderately acidic to strongly alkaline) is within the natural range of
 the groundwater and the down gradient landuse is restricted to National Park activities.
- surface water release or groundwater movement from the proposed dredge pond is not expected to result in deterioration (or improvement) of the quality of the water in Blue Angle Creek. The installation and sampling of new monitoring bores, together with the continued sampling and testing of existing groundwater and surface water sampling sites, will provide on-going auditing of any effects of the extraction process.
- potential acid sulphate soil conditions exist within the sand resource and are locally exceed the action criteria. Similar conditions probably also in the included Unit 3 clays and some sections of the Unit 5 clays forming the base level of the deposit.
- as a consequence of the exceedances of the action criteria, an acid sulphate soil management plan will be required. However, the available data on groundwater indicators are not strongly indicative of actual sulphate soil conditions nor does the dredge pond monitoring indicate that the extraction process within Units 1 and 2 is resulting in oxidation of significant amounts of pyritic material.

DOUGLAS PARTNERS PTY LTD

Reviewed by

G R Wilson

Principal

Dr T J Wiesner Principal

APPENDIX A Notes Relating to this Report Results of Field Work

Douglas Partners Geotechnics · Environment · Groundwater

NOTES RELATING TO THIS REPORT

Introduction

These notes have been provided to amplify the geotechnical report in regard to classification methods, specialist field procedures and certain matters relating to the Discussion and Comments section. Not all, of course, are necessarily relevant to all reports.

Geotechnical reports are based on information gained from limited subsurface test boring and sampling, supplemented by knowledge of local geology and experience. For this reason, they must be regarded as interpretive rather than factual documents, limited to some extent by the scope of information on which they rely.

Description and Classification Methods

The methods of description and classification of soils and rocks used in this report are based on Australian Standard 1726, Geotechnical Site Investigations Code. In general, descriptions cover the following properties strength or density, colour, structure, soil or rock type and inclusions.

Soil types are described according to the predominating particle size, qualified by the grading of other particles present (eg. sandy clay) on the following bases:

Soil Classification	Particle Size
Clay	less than 0.002 mm
Silt	0.002 to 0.06 mm
Sand	0.06 to 2.00 mm
Gravel	2.00 to 60.00 mm

Cohesive soils are classified on the basis of strength either by laboratory testing or engineering examination. The strength terms are defined as follows.

	Undrained
Classification	Shear Strength kPa
Very soft	less than 12
Soft	12—25
Firm	25—50
Stiff	50—100
Very stiff	100—200
Hard	Greater than 200

Non-cohesive soils are classified on the basis of relative density, generally from the results of standard penetration tests (SPT) or Dutch cone penetrometer tests (CPT) as below:

Relative Density	SPT "N" Value (blows/300 mm)	CPT Cone Value (q _c — MPa)
Very loose	less than 5	less than 2
Loose	5—10	2—5
Medium dense	10—30	5—15
Dense	30—50	15—25

Very dense greater than 50 greater than 25 Rock types are classified by their geological names. Where relevant, further information regarding rock classification is given on the following sheet.

Sampling

Sampling is carried out during drilling to allow engineering examination (and laboratory testing where required) of the soil or rock.

Disturbed samples taken during drilling provide information on colour, type, inclusions and, depending upon the degree of disturbance, some information on strength and structure.

Undisturbed samples are taken by pushing a thinwalled sample tube into the soil and withdrawing with a sample of the soil in a relatively undisturbed state. Such samples yield information on structure and strength, and are necessary for laboratory determination of shear strength and compressibility. Undisturbed sampling is generally effective only in cohesive soils.

Details of the type and method of sampling are given in the report.

Drilling Methods.

The following is a brief summary of drilling methods currently adopted by the Company and some comments on their use and application.

Test Pits — these are excavated with a backhoe or a tracked excavator, allowing close examination of the in-situ soils if it is safe to descent into the pit. The depth of penetration is limited to about 3 m for a backhoe and up to 6 m for an excavator. A potential disadvantage is the disturbance caused by the excavation.

Large Diameter Auger (eg. Pengo) — the hole is advanced by a rotating plate or short spiral auger, generally 300 mm or larger in diameter. The cuttings are returned to the surface at intervals (generally of not more than 0.5 m) and are disturbed but usually unchanged in moisture content. Identification of soil strata is generally much more reliable than with continuous spiral flight augers, and is usually supplemented by occasional undisturbed tube sampling.

Continuous Sample Drilling — the hole is advanced by pushing a 100 mm diameter socket into the ground and withdrawing it at intervals to extrude the sample. This is the most reliable method of drilling in soils, since moisture content is unchanged and soil structure, strength, etc. is only marginally affected.

Continuous Spiral Flight Augers — the hole is advanced using 90—115 mm diameter continuous spiral flight augers which are withdrawn at intervals to allow



sampling or in-situ testing. This is a relatively economical means of drilling in clays and in sands above the water table. Samples are returned to the surface, or may be collected after withdrawal of the auger flights, but they are very disturbed and may be contaminated. Information from the drilling (as distinct from specific sampling by SPTs or undisturbed samples) is of relatively lower reliability, due to remoulding, contamination or softening of samples by ground water.

Non-core Rotary Drilling — the hole is advanced by a rotary bit, with water being pumped down the drill rods and returned up the annulus, carrying the drill cuttings. Only major changes in stratification can be determined from the cuttings, together with some information from 'feel' and rate of penetration.

Rotary Mud Drilling — similar to rotary drilling, but using drilling mud as a circulating fluid. The mud tends to mask the cuttings and reliable identification is again only possible from separate intact sampling (eg. from SPT).

Continuous Core Drilling — a continuous core sample is obtained using a diamond-tipped core barrel, usually 50 mm internal diameter. Provided full core recovery is achieved (which is not always possible in very weak rocks and granular soils), this technique provides a very reliable (but relatively expensive) method of investigation.

Standard Penetration Tests

Standard penetration tests (abbreviated as SPT) are used mainly in non-cohesive soils, but occasionally also in cohesive soils as a means of determining density or strength and also of obtaining a relatively undisturbed sample. The test procedure is described in Australian Standard 1289, "Methods of Testing Soils for Engineering Purposes" — Test 6.3.1.

The test is carried out in a borehole by driving a 50 mm diameter split sample tube under the impact of a 63 kg hammer with a free fall of 760 mm. It is normal for the tube to be driven in three successive 150 mm increments and the 'N' value is taken as the number of blows for the last 300 mm. In dense sands, very hard clays or weak rock, the full 450 mm penetration may not be practicable and the test is discontinued.

The test results are reported in the following form.

 In the case where full penetration is obtained with successive blow counts for each 150 mm of say 4, 6 and 7

• In the case where the test is discontinued short of full penetration, say after 15 blows for the first 150 mm and 30 blows for the next 40 mm

as 15, 30/40 mm.

The results of the tests can be related empirically to the engineering properties of the soil.

Occasionally, the test method is used to obtain

samples in 50 mm diameter thin walled sample tubes in clays. In such circumstances, the test results are shown on the borelogs in brackets.

Cone Penetrometer Testing and Interpretation

Cone penetrometer testing (sometimes referred to as Dutch cone — abbreviated as CPT) described in this report has been carried out using an electrical friction cone penetrometer. The test is described in Australian Standard 1289, Test 6.4.1.

In the tests, a 35 mm diameter rod with a cone-tipped end is pushed continuously into the soil, the reaction being provided by a specially designed truck or rig which is fitted with an hydraulic ram system. Measurements are made of the end bearing resistance on the cone and the friction resistance on a separate 130 mm long sleeve, immediately behind the cone. Transducers in the tip of the assembly are connected by electrical wires passing through the centre of the push rods to an amplifier and recorder unit mounted on the control truck.

As penetration occurs (at a rate of approximately 20 mm per second) the information is plotted on a computer screen and at the end of the test is stored on the computer for later plotting of the results.

The information provided on the plotted results comprises: —

- Cone resistance the actual end bearing force divided by the cross sectional area of the cone expressed in MPa.
- Sleeve friction the frictional force on the sleeve divided by the surface area expressed in kPa.
- Friction ratio the ratio of sleeve friction to cone resistance, expressed in percent.

There are two scales available for measurement of cone resistance. The lower scale (0-5 MPa) is used in very soft soils where increased sensitivity is required and is shown in the graphs as a dotted line. The main scale (0-50 MPa) is less sensitive and is shown as a full line.

The ratios of the sleeve friction to cone resistance will vary with the type of soil encountered, with higher relative friction in clays than in sands. Friction ratios of 1%—2% are commonly encountered in sands and very soft clays rising to 4%—10% in stiff clays.

In sands, the relationship between cone resistance and SPT value is commonly in the range:—

 q_c (MPa) = (0.4 to 0.6) N (blows per 300 mm)

In clays, the relationship between undrained shear strength and cone resistance is commonly in the range: $q_c = (12 \text{ to } 18) c_u$

Interpretation of CPT values can also be made to allow estimation of modulus or compressibility values to allow calculation of foundation settlements.

Inferred stratification as shown on the attached reports is assessed from the cone and friction traces and from experience and information from nearby boreholes, etc. This information is presented for general guidance, but must be regarded as being to some extent interpretive. The test method provides a continuous profile of engineering properties, and where precise information on



soil classification is required, direct drilling and sampling may be preferable.

Hand Penetrometers

Hand penetrometer tests are carried out by driving a rod into the ground with a falling weight hammer and measuring the blows for successive 150 mm increments of penetration. Normally, there is a depth limitation of 1.2 m but this may be extended in certain conditions by the use of extension rods.

Two relatively similar tests are used.

- Perth sand penetrometer a 16 mm diameter flatended rod is driven with a 9 kg hammer, dropping 600 mm (AS 1289, Test 6.3.3). This test was developed for testing the density of sands (originating in Perth) and is mainly used in granular soils and filling.
- Cone penetrometer (sometimes known as the Scala Penetrometer) — a 16 mm rod with a 20 mm diameter cone end is driven with a 9 kg hammer dropping 510 mm (AS 1289, Test 6.3.2). The test was developed initially for pavement subgrade investigations, and published correlations of the test results with California bearing ratio have been published by various Road Authorities.

Laboratory Testing

Laboratory testing is carried out in accordance with Australian Standard 1289 "Methods of Testing Soil for Engineering Purposes". Details of the test procedure used are given on the individual report forms.

Bore Logs

The bore logs presented herein are an engineering and/or geological interpretation of the subsurface conditions, and their reliability will depend to some extent on frequency of sampling and the method of drilling. Ideally, continuous undisturbed sampling or core drilling will provide the most reliable assessment, but this is not always practicable, or possible to justify on economic grounds. In any case, the boreholes represent only a very small sample of the total subsurface profile.

Interpretation of the information and its application to design and construction should therefore take into account the spacing of boreholes, the frequency of sampling and the possibility of other than 'straight line' variations between the boreholes.

Ground Water

Where ground water levels are measured in boreholes, there are several potential problems;

- In low permeability soils, ground water although present, may enter the hole slowly or perhaps not at all during the time it is left open.
- A localised perched water table may lead to an erroneous indication of the true water table.

- Water table levels will vary from time to time with seasons or recent weather changes. They may not be the same at the time of construction as are indicated in the report.
- The use of water or mud as a drilling fluid will mask any ground water inflow. Water has to be blown out of the hole and drilling mud must first be washed out of the hole if water observations are to be made.

More reliable measurements can be made by installing standpipes which are read at intervals over several days, or perhaps weeks for low permeability soils. Piezometers, sealed in a particular stratum, may be advisable in low permeability soils or where there may be interference from a perched water table.

Engineering Reports

Engineering reports are prepared by qualified personnel and are based on the information obtained and on current engineering standards of interpretation and analysis. Where the report has been prepared for a specific design proposal (eg. a three storey building), the information and interpretation may not be relevant if the design proposal is changed (eg. to a twenty storey building). If this happens, the Company will be pleased to review the report and the sufficiency of the investigation work.

Every care is taken with the report as it relates to interpretation of subsurface condition, discussion of geotechnical aspects and recommendations or suggestions for design and construction. However, the Company cannot always anticipate or assume responsibility for:

- unexpected variations in ground conditions the potential for this will depend partly on bore spacing and sampling frequency
- changes in policy or interpretation of policy by statutory authorities
- the actions of contractors responding to commercial pressures.

If these occur, the Company will be pleased to assist with investigation or advice to resolve the matter.

Site Anomalies

In the event that conditions encountered on site during construction appear to vary from those which were expected from the information contained in the report, the Company requests that it immediately be notified. Most problems are much more readily resolved when conditions are exposed than at some later stage, well after the event.

Reproduction of Information for Contractual Purposes

Attention is drawn to the document "Guidelines for the Provision of Geotechnical Information in Tender Documents", published by the Institution of Engineers,



Australia. Where information obtained from this investigation is provided for tendering purposes, it is recommended that all information, including the written report and discussion, be made available. In circumstances where the discussion or comments section is not relevant to the contractual situation, it may be appropriate to prepare a specially edited document. The Company would be pleased to assist in this regard and/or to make additional report copies available for contract purposes at a nominal charge.

Site Inspection

The Company will always be pleased to provide engineering inspection services for geotechnical aspects of work to which this report is related. This could range from a site visit to confirm that conditions exposed are as expected, to full time engineering presence on site.

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Cone Penetrometer Test

The Dutch cone penetrometer test (CPT) is increasing in importance as a means of supplementing or replacing conventional borehole drilling and sampling. These notes describe current techniques and equipment, and illustrate some of the methods of interpretation of test results.

GENERAL PRINCIPLES

The cone penetrometer test, developed initially in Holland, is now standardised throughout the world as a method of site investigation, predominantly used in alluvial soils.

The test utilises a 35mm dia. cone with a following friction sleeve which is pushed into the soil by hydraulic thrust. Push rods, of the same diameter, are added at 1m intervals and penetration to depths of 30 - 50m can be achieved if conditions are suitable.

Measurements are made of the resistance to penetration of both the cone and the friction sleeve and are expressed in terms of end bearing pressure on the cone and average shaft friction on the sleeve (in MPa and kPa, respectively). The ratio between the frictional resistance and the cone resistance (friction ratio) is generally low in sands and high in clays, thus providing an interpretive method of determining soil stratification. The cone resistance value provides a continuous measure of soil strength or density.

The test procedure is covered by Australian Standard AS 1289 ("Testing Soils for Engineering Purposes") — Section F5.



METHOD OF OPERATION

Up until the early 80's, "mechanical" equipment was used, with central push rods to actuate the cone, with forces and measured by pressure gauges in surface mounted hydraulic cylinders.

Today's equipment utilises electronic methods with:

• strain gauges in the cone and friction sleeve, connected by wiring through the push rods to the surface

• a site monitor and computer to capture the data — results are displayed on a screen

replay of results on an office plotter

• an inclinometer in the cone to measure deviations from vertical.

Testing is preferably carried out from a purpose designed ballasted truck-mounted unit, with sufficient thrust capacity to ensure penetration of dense layers. A unit with about 15 tonnes thrust is normally adequate for most situations.

Testing may also be carried out from the feed system of drill rigs, but unless special anchoring is undertaken, the available thrust capacity of 2 - 3 tonnes is not sufficient to penetrate dense sand layers or extensive zones of stiff clays.

The 35mm cone and friction sleeve are attached to push rods of the same diameter and pushed into the soil by a hydraulic ram. New rods, through which the electrical data cable has been pre-threaded, are added after each 1m of penetration. The rate of penetration is set at 20mm/second and strain gauge signals are sampled by the surface monitor at 20mm intervals. Digital results for cone resistance, shaft adhesion and friction ratio are displayed on the screen of the on-site computer and on completion of each test, the operator can view a graph of the results. These are stored on a floppy disk for later office replay on a plotter.

ADVANTAGES AND APPLICATIONS

Advantages of the cone penetrometer test in engineering site investigations are:

• reliable information, not subject to operator techniques and with minimal need for on-site supervision

• continuous (rather than intermittent) measure of density or strength

 extensive data available from previous experience for analysis and interpretation of results

- immediate charting of results
- high output 80 to 120m per day

• low cost technique, less than alternative borehole drilling, sampling and in-situ testing

• accurate measurement of stratification depths and recording of thin layers, often missed in borehole investigations.

The test system is most suited for alluvial soils and depths of up to 30 – 50m are possible, depending on ground conditions.

The test is not suitable in very gravelly soils or residual soils which may contain fragments or weak rock zones which impede or deflect the cone.

On sites containing rubble or rock filling, initial assistance may be required from a drilling rig to penetrate the surface.

The information obtained from the test can be used for design of shallow foundations, pile systems and retaining structures and the estimation of settlements of compressible soils.

CONDUCTIVITY MEASUREMENTS FOR GROUNDWATER

Cone technology can be used also in groundwater investigations. The conductivity cone can measure variations in soil conductivity with depth, making it an ideal tool in the investigation of saline groundwater or pollutant plumes.



COMPANY FACILITIES

Company inhouse facilities for cone penetrometer testing include:

- cones and monitor units with automatic capture of results
- incorporation of an inclinometer in the cones to ensure verticality
- truck-mounted, ballasted, customdesigned deployment units attached to Sydney and Brisbane offices
- trailer mounted rig and light hand operated equipment for confined areas
- heavy-duty drilling rig with a separate jacking frame in the centre of the unit to allow high capacity testing
- conductivity cone for groundwater investigations.

These facilities are continually being upgraded to keep abreast of technology improvements.
INTERPRETATION OF RESULTS

A great deal of research and investigation has been carried out to correlate the results of CPT's with other physical soil properties.

DETERMINATION OF SOIL TYPE

The ratio of sleeve friction to cone resistance (friction ratio — expressed as %) is used as an indication of the soil type being probed. In sands, friction ratios are generally low and of the order of 1 - 2%. In clays (with much higher shaft friction), the ratio may range up to 6 - 8%.

A chart which allows assessment of soil type is given below:



With the above, the cone-friction sleeve results allow an accurate assessment of layer thickness (stratigraphy) within a profile. The test method will provide a very much more accurate delineation of layers than is possible with conventional drilling and sampling, where information is only obtained intermittently. The cone test will frequently pick up thin layers which may be unnoticed in a conventional borehole investigation.

DETERMINATION OF SOIL STRENGTH

Soil strength is usually determined from the cone resistance values. The normally accepted relationship for clays, linking undrained shear strength with cone resistance is:

СЦ		= (q _C -σ)/k	
where	q _C	= cone resistance	
	Cu	 undrained cohesion 	
	σ	= effective stress	
	k	= 12 - 15	

For sands, there are a number of methods of linking cone resistance values with friction angle (ϕ). Interpretation must normally take into account the overburden pressure, but as a guide, for a depth of around 5m, the following is an approximate correlation:

q _C (MPa)	1	4	12	30
φ (degrees)	28	32	36	40

CORRELATION WITH SPT VALUES

There have been many measurements to correlate standard penetration test values with CPT's. For medium to coarse sands, the correlation is:

 q_{C} (MPa) = (0.4 to 0.6)N (blows per 300mm)

For fine sands, the multiplier is 0.3 to 0.45.

MODULUS AND DEFORMATION VALUES

Published correlations linking cone resistance and modulus or compressibility values indicate a relatively wide range of values. This is probably due to inherent soil variability, time effects and the fact that modulus in any case is not uniquely dependent on strength or density.

For clays, the published correlations are mainly in the range $E = (3 \text{ to } 7)q_C$ ($E = Young's \mod ulus$). Values of the multiplier of around 3 - 5 are probably most appropriate for soft, normally consolidated clays, with values of 5 - 7 for stiffer and slightly over-consolidated clays. Values in excess of 10 may be appropriate for heavily over-consolidated or residual clays.

For sands, published correlations are mainly in the range $E = (1.5 \text{ to } 5)q_C$, with values towards the upper end of the range being appropriate in most circumstances.

FOUNDATION DESIGN

A number of methods are available which utilise cone results directly for the design of both shallow and deep foundations.

The cone is often considered as a model "pile" and for pile design, corrections are needed for the scale effect and the fact that static rather than "quasi static" bearing capacity is measured. More detailed information is given in the references.

REFERENCES

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Stratification is inferred from friction ratio and from previous experience and knowledge 1213 150



Sydney, Newcastle, Brisbane, Melbourne, Perth, Wyong, Campbelitown, Townsville, Cairns, Wollongong, Darwin

CLIENT: CLEARY BROS (BOMBO) PTY LTD

PROJECT: PROPOSED NORTHERN EXTENSION

LOCATION: GERROA & BEACH ROADS, GERROA

PROJECT No: 37673

CPT 101

 Page 1 of 1

 DATE
 9/12/04

 SURFACE RL: 4.9

8 10

Depth

(m)

0

2

3

5

6

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16

17

18

19



REMARKS: HOLE COLLAPSE AT 3.35m E 280492 N 1150028 (ISG) : E 297594 N 6149316 (MGA)



File: N:\GEOTECHNICAL PROJECTS\376\37673\CPT files\37673101.CP5
Cone ID: H3 Type: 2 Standard



CLIENT: CLEARY BROS (BOMBO) PTY LTD

PROJECT: PROPOSED NORTHERN EXTENSION

LOCATION: GERROA & BEACH ROADS, GERROA

PROJECT No: 37673

Page 1 of 1

DATE 9/12/04
SURFACE RL: 5.2



		Friction R _f (%)	i Rati	0		
Soil Behaviour Type		0 2	4	6 L	8 10	Depth (m)
SAND: Medium Dense to	1		-	<u> </u>	<u> </u>	- 0
Dense						
		4		-		- 1
						5
						-2
	3.09					- 3
SAND with some GRAVELLY SAND: Medium Dense to	0.00					
Very Dense		}	_			-4
					1	
						-5
		2				- 6
		$\left\{ \right\}$				- 7
		$ \langle $				
		{				- 8
						, U
		{				
		}				3
CAND: Madium Damas in	9.89	5				10
Very Dense						
						14
		{				- 11
						10
						. 12
						43
						- 13
					-	
		[{				14
						15
	15.0					15
						16
						- 17
						- 18
						13
						10
						12
						- 20

REMARKS: HOLE COLLAPSE AT 4.2m E 280439 N 1149951 (ISG) : E 297542 N 6149238 (MGA)



 File: N:\GEOTECHNICAL PROJECTS\\376\376\37673\CPT files\\37673102.CP5

 Cone ID: H3
 Type: 2 Standard



CLIENT: CLEARY BROS (BOMBO) PTY LTD

PROJECT: PROPOSED NORTHERN EXTENSION

LOCATION: GERROA & BEACH ROADS, GERROA

PROJECT No: 37673

CPT 103

Page 1 of 1 DATE 9/12

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DATE 9/12/04 SURFACE RL: 5.3



REMARKS: HOLE COLLAPSE AT 4.65m E 280331 N 1149932 (ISG) : E 297435 N 6149217 (MGA)



 File: N:\GEOTECHNICAL PROJECTS\376\37673\CPT files\37673103,CP5

 Cone ID: H3
 Type: 2 Standard

CLIENT: CLEARY BROS (BOMBO) PTY LTD

PROJECT: PROPOSED NORTHERN EXTENSION

LOCATION: GERROA & BEACH ROADS, GERROA

PROJECT No: 37673

CPT 104

Page 1 of 1 DATE 9/12/04

SURFACE RL: 5.8



REMARKS: HOLE COLLAPSE AT 4.15m E 280314 N 1149844 (ISG) : E 297419 N 6149129 (MGA)



 File: N:\GEOTECHNICAL PROJECTS\376\37673\CPT files\37673104.CP5

 Cone ID: H3
 Type: 2 Standard



CLIENT: CLEARY BROS (BOMBO) PTY LTD

PROJECT: PROPOSED NORTHERN EXTENSION

LOCATION: GERROA & BEACH ROADS, GERROA

PROJECT No: 37673

500

400

Page 1 of 1 DATE 9/12/04

SURFACE RL: 5.1





REMARKS: HOLE COLLAPSE AT 4.2m E 280217 N 1149810 (ISG) : E 297326 N 6148968 (MGA)



File: D:\37673\CPT files\37673105.CP5 Cone ID: H3 Type: 2 Standard



CLIENT: CLEARY BROS (BOMBO) PTY LTD

PROJECT: PROPOSED NORTHERN EXTENSION

LOCATION: GERROA & BEACH ROADS, GERROA

PROJECT No: 37673

DATE 9/12/04

SURFACE RL: 6.8



		Friction Rr (%)	Rat	io			
Soil Behaviour Type	(2	4	6 -	8	10 J	Depth (m)
SAND with some GRAVELLY SAND: Loose to Dense		5	-				0
							- 1
							-2
							- 3
SAND with some GRAVELLY SAND: Dense to Very Dense	3.78	}				-	- 4
							-5
	6.58						- 6
GRAVELLY SAND: Dense to Very Dense		5					-7
							- 8
		}					- 9
SAND: Medium Dense	10.18	<u>}</u>	_			_	- 10
							- 11
							- 12
							- 13
	14.5					_	- 14
SILTY CLAY: Very Stiff	15.0			-			- 15
							- 16
						_	- 17
						_	- 18
						_	- 19
]						- 20

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REMARKS: HOLE COLLAPSE AT 4.3m E 280156 N 1149686 (ISG) : E 297265 N 6148968 (MGA)



 File: N:\GEOTECHNICAL PROJECTS\376137673\CPT files\37673106.CP5

 Cone ID: H3
 Type: 2 Standard

CLIENT: CLEARY BROS (BOMBO) PTY LTD

PROJECT: PROPOSED NORTHERN EXTENSION

LOCATION: GERROA & BEACH ROADS, GERROA

PROJECT No: 37673

CPT 107

SURFACE RL: 4.1

(()) Douglas Partners

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Page 1 of 1 DATE 9/12/04



REMARKS: HOLE COLLAPSE AT 1.85m E 280057 N 1149734 (ISG) : E 297165 N 6149014 (MGA)



 File: N:\GEOTECHNICAL PROJECTS\376\37673\CPT files\37673107.CP5

 Cone ID: H3
 Type: 2 Standard

CLIENT: CLEARY BROS (BOMBO) PTY LTD

PROJECT: PROPOSED NORTHERN EXTENSION

LOCATION: GEROA & BEACH ROADS, GERROA

PROJECT No: 37673

CPT 108

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DATE 9/12/04
SURFACE RL: 6.8



REMARKS: HOLE COLLAPSE AT 4.3m E 280065 N 1149606 (ISG) : E 297175 N 6148886



 File: N:\GEOTECHNICAL PROJECTS\376\37673\CPT files\37673108.CP5

 Cone ID: H3
 Type: 2 Standard



CLIENT: CLEARY BROS (BOMBO) PTY LTD

PROJECT: PROPOSED NORTHERN EXTENSION

LOCATION: GERROA & BEACH ROADS, GERROA

PROJECT No: 37673

Page 1 of 1 DATE 9/12/04

SURFACE RL: 2.5

1

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REMARKS: HOLE COLLAPSE AT 0.65m E 279893 N 1149711 (ISG) : E 297001 N 6148987 (MGA)



File: N:\GEOTECHNICAL PROJECTS\376\37673\CPT files\37673109.CP5 Type: 2 Standard Cone ID; H3



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PROJECT: PROPOSED NORTHERN EXTENSION

LOCATION: GERROA & BEACH ROADS, GERROA

PROJECT No: 37673

CPT 110

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 DATE
 9/12/04

 SURFACE RL:
 2.4



REMARKS: HOLE COLLAPSE AT 1.3m E 279924 N 1149617 (ISG) : E 297034 N 6148894 (MGA)



 File: N:\GEOTECHNICAL PROJECT\$\376\37673\CPT files\37673110.CP5

 Cone ID: H3
 Type: 2 Standard



CLIENT: CLEARY BROS (BOMBO) PTY LTD

PROJECT: PROPOSED NORTHERN EXTENSION

GERROA & BEACH ROADS, GERROA

LOCATION:

PROJECT No: 37673

CPT 111

Page 1 of 1 DATE 9/12/04

SURFACE RL: 1.2



REMARKS: HOLE COLLAPSE AT 0.7m E 279646 N 1149509 (ISG) : E 296758 N 6142780 (MGA)



 File:
 D:\37673\CPT files\37673111.CP5

 Cone ID:
 H3
 Type: 2 Standard



 SURFACE LEVEL:
 4.5m

 EASTING:
 280456

 NORTHING:
 1149997

 DIP/AZIMUTH:
 90°/-

BORE No: 201 PROJECT No: 37673 DATE: 20 Dec 04 SHEET 1 OF 2

		_	Description	Degree of Weathering	0	Rock Strength	Fracture	Discontinuities	Sa	mpling &	In Situ Testing
RL	D	epth (m)	n of		Log		Spacing (m)	B - Bedding J - Joint	be		Test Results
			Strata	HW MW SS MW SS F			0.01 0.10 1.00 1.00	S - Shear D - Drill Break	4	ပိမ္ရွိဆို ိ	Comments
	فسيقيد فيسقيه فيعاسيه والمراجع	0.	5 SILTY SAND - grey brown silty sand, rootlets, damp (topsoil) 5 SILTY SAND - very loose, orange brown silty sand, rootlets, damp						А		
	- 1				 				s		2,2,2 N = 4
		2	0		- [-] -				A		
			SAND - loose, pale orange brown fine grained sand, rootlets, damp						s		2,3,3 N = 6
	-3		- medium dense sand, moist to wet						s		3,7,12 N = 19
	-4		- medium dense, orange grey fine to medium grained sand, wet						s		6,7,9 N = 16
		5.	.0 GRAVELLY SAND - dense, orange grey coarse grained sand and subrounded quartz, siltstone, gravel with shell fragments, saturated						S		6,16,30 N = 46
	6 				0.000				S		30 for 150mm,-,- refusal
	-7 - - - - -		- very dense sand		0,0,0				s		19,23,28 N = 51
	-8		- grey gravelly sand		0,000				s		11,14,19 N = 33

RIG: Scout

CLIENT:

PROJECT:

Cleary Bros (Bombo) Pty Ltd

Proposed Northern Extension

LOCATION: Gerroa Sand Quarry, Gerroa

DRILLER: Driver

LOGGED: L McKenzie

CASING: to 6.0m

TYPE OF BORING:Spiral flight auger to 5.5m; Rotary to 13.5mWATER OBSERVATIONS:Free groundwater observed at 3.5mREMARKS:E 297558 N 6149284 (MGA)

CHECKED Initials: Show Date: 7/01 Douglas Partners Geotechnics · Environment · Groundwater
Initia Dati

SURFACE LEVEL: 4.5m EASTING: 280456 NORTHING: 1149997 DIP/AZIMUTH: 90°/--

BORE No: 201 PROJECT No: 37673 DATE: 20 Dec 04 SHEET 2 OF 2

	Description	Degree of Weathering	lic	Rock Strength	Fracture	Discontinuities	Sa	mpling &	In Situ Testing
교 Dep (m	n) of	, , , , , , , , , , , , , , , , , , ,	irapt Log		(m)	B - Bedding J - Joint	be	9% 0%	Test Results
	Strata	₩ ₩ ₩ ₩ % 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8			0.10	S - Shear D - Drill Break	F	ပစ္စိုင်း	Comments
	9.25 SAND - dense, dark grey medium to coarse grained sand, saturated		0				s		25,25,16 N = 41
- 10									
							s		N = 41
-							s	-	13,18,26 N = 44
- 12							s		16,24,25 N = 49
- 13	12.8 SILTY SAND - medium dense, silty sand						s		10,11,13 N = 24
- 14 - 15 - 16 - 17	^{13.5} Bore discontinued at 13.5m (refusal on possible weathered rock or very dense sand)								

RIG: Scout **DRILLER:** Driver TYPE OF BORING: Spiral flight auger to 5.5m; Rotary to 13.5m WATER OBSERVATIONS: Free groundwater observed at 3.5m REMARKS: E 297558 N 6149284 (MGA)

Core drilling

Cleary Bros (Bombo) Pty Ltd

Proposed Northern Extension

LOCATION: Gerroa Sand Quarry, Gerroa

CLIENT:

PROJECT:

LOGGED: L McKenzie

CASING: to 6.0m

SAMPLING & IN SITU TESTING LEGEND pp Pocket penetrometer (kPa) PID Photo ionisation detector S Standard penetration test mm dia.) PL Point load strength is(50) MPa V Shear Vane (kPa) Water seep T Water level Auger sample Disturbed sample Bulk sample Tube sample (x mm dia.) Water sample Core dillion ADBU,WC Initials: Lyhiw Date: 1/01





CLIENT: PROJECT:

Cleary Bros (Bombo) Pty Ltd Proposed Northern Extension LOCATION: Gerroa Sand Quarry, Gerroa

SURFACE LEVEL: 5.1m EASTING: 280320 NORTHING: 1149878 DIP/AZIMUTH: 90°/--

BORE No: 202 PROJECT No: 37673 DATE: 21 Dec 04 SHEET 1 OF 2

Γ		Description	Degree of Weathering	<u>u</u>	Rock Strength	Fracture	Discontinuities	Sar	mpling & I	In Situ Testing
Ч	Depth (m)	۱ of	, , out is in g	Log		Spacing (m)	B - Bedding J - Joint	g	e», C	Test Results
		Strata	H M M M M M M M M M M M M M M M M M M M	σ	Low High	0.01 0.105 0.10 1.00	S - Shear D - Drill Break	2	S B R &	č Comments
	0	SILTY SAND - grey brown silty sand (topsoil) 5 SILTY SAND - very loose, orange brown fine grained silty sand, rootlets, damp						A		
	- - - - -							A,S A		2,2,2 N = 4
	2	- pale orange grey						s		1,1,1 N = 2
	-3	- pale orange grey						s		-,1,3
	- 3.	³ SAND - medium dense, pale orange grey fine to medium grained sand, trace shell fragments, wet	anna bann sao sao sa							N = 4
	-4							s		2,7,10 N = 17
	-5		And Long Went Annu Annu Annu Annu Annu Annu Annu An					s		4,11,17 N = 28
	-6	 very dense, orange grey fine to medium grained sand with some shells, shell fragments, saturated 	A SAA SAA SAA SAA SAA SAA SAA SAA SAA S					s		16,25,25 N = 50
	-7	- slightly coarse grained	AND AND AND AND			anna anna anna anna		s		15,26,25 N = 51
	-8	⁵ GRAVELLY SAND - grey coarse grained sand and subrounded quartz, siltstone gravel with shells and shell fragments, saturated	Name Nam Name Name	0000		And the state of t		S		18,30,- refusal
	- - - - -			0 0 0						

RIG: Scout **DRILLER:** Driver TYPE OF BORING: Spiral flight auger to 5.5m; Rotary to 15.2m WATER OBSERVATIONS: Free groundwater observed at 3.8m **REMARKS:** E 297425 N 6149163 (MGA)

SAMPLING & IN SITU TESTING LEGEND pp Pocket penetrometer (kPa) PID Photo ionisation detector S Standard penetration test mm dia.) PL Point load strength Is(50) MPa V Shear Vane (kPa) D Water seep ¥ Water level CHECKED SAMPI Auger sample Disturbed sample Bulk sample Tube sample (x mm dia.) Water sample Core drilling ADBU,WC Initials: Sho う) Date: Û

LOGGED: L McKenzie

CASING: to 8.5m

Douglas Partners Geotechnics · Environment · Groundwater

 SURFACE LEVEL: 5.1m

 EASTING:
 280320

 NORTHING:
 1149878

 DIP/AZIMUTH:
 90°/-

BORE No: 202 PROJECT No: 37673 DATE: 21 Dec 04 SHEET 2 OF 2

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	- 9.1	SAND - dense, dark grey medium												_			7.4.5
		to coarse grained sand, trace			·· `·									5			N = 9
	-	gravel, possibly slightly organic, saturated								1							
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RIG: ScoutDRILLER: DriverTYPE OF BORING: Spiral flight auger to 5.5m; Rotary to 15.2mWATER OBSERVATIONS: Free groundwater observed at 3.8mREMARKS:E 297425 N 6149163 (MGA)

CLIENT:

PROJECT:

Cleary Bros (Bombo) Pty Ltd

Proposed Northern Extension

LOCATION: Gerroa Sand Quarry, Gerroa

	,	SAMPLING & IN SITU	TE	STING LEGEND
	A	Auger sample	DD	Pocket penetrometer (kPa)
	D	Disturbed sample	ΡΊD	Photo ionisation detector
Ì	в	Bulk sample	S	Standard penetration test
1	U,	Tube sample (x mm dia.)	ΡL	Point load strength Is(50) MPa
	Ŵ	Water sample	V.	Shear Vane (kPa)
ĺ	С	Core drilling		Water seep Water level



LOGGED: L McKenzie



CASING: to 8.5m

SURFACE LEVEL: 6.0m EASTING: 280219 NORTHING: 1149752 DIP/AZIMUTH: 90°/--

BORE No: 203 PROJECT No: 37673 DATE: 22 Dec 04 SHEET 1 OF 2

		Description	Degree of	0	Rock	Fracture	Discon	tinuities	Sa	molin	n & I	n Situ Testing
님	Depth	of	Weathering	og og	Strengtn	Spacing	D. Deddies	1 12:24	<u>л</u>	n %		Test Results
[(m)	Strata	A A A o ⊮	5 -	x Low	(m) 5 89 88	B - Bedding S - Shear	J - Joint D - Drill Break	ŢYP	ç ç	gg %	& Commonts
		SILTY SAND - grey brown silty		· j · j ·	<u> </u>		•					Commenta
	U.2	SILTY SAND - very loose, orange		· · ·								
	-	brown silty sand, rootlets, damp to moist		· [•]•					А			
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	-6 6.0											
	-	SAND - loose, orange brown fine grained sand, wet							s			4,5,4
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ŀ		subrounded quartz, siltstone gravel		0	anala anala Anala anala Anala anala							
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RIG: Scout

CLIENT:

PROJECT:

Cleary Bros (Bombo) Pty Ltd

Proposed Northern Extension

LOCATION: Gerroa Sand Quarry, Gerroa

DRILLER: Driver

LOGGED: L McKenzie

CASING: to 5.5m

 TYPE OF BORING:
 Spiral flight auger to 5.5m; Rotary to 17.5m

 WATER OBSERVATIONS:
 Free groundwater observed at 4.0m

 REMARKS:
 E 297 326 N 6149035 (MGA)

Douglas Partners
にてん

280219

1149752

Cleary Bros (Bombo) Pty Ltd SURFACE LEVEL: 6.0m **Proposed Northern Extension** EASTING: LOCATION: Gerroa Sand Quarry, Gerroa NORTHING: DIP/AZIMUTH: 90°/--

CLIENT:

PROJECT:

BORE No: 203 PROJECT No: 37673 DATE: 22 Dec 04 SHEET 2 OF 2

		Description	Degree of Weatherin	2	Rock Strength	Fracture	Discontinuities	Sa	mplir	ng &	In Situ Testing
R	Depth (m)	of		iraph Log		Spacing (m)	B - Bedding J - Joint	ъ	sre %	g.	Test Results
		Strata	EW MW Fs	Ξ Ξ	Ex Low Medi Hgh Ex Hgh	0.01	S - Shear D - Drill Break	ту	ပိမ္ရွိ	Υ,	Comments
	-	Coarse grained sand and subrounded quartz, siltstone gravel with shells and shell fragments, saturated (continued) - loose sand	10 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1					S			5,3,5 N = 8
	- 10 - 10 	 medium dense, dark brown and grey sand, trace organics, saturated 		000				s			14,8,12 N = 20
	-11 11.0	SAND - dense, dark grey brown medium to coarse grained sand, trace organics, saturated		q				s			11,16,20 N = 36
	- 12							s			21,25 for 80mm,- refusal
	- 13	- trace gravel and shell fragments						s			20,30 for 100mm,- refusal
	- 14	- grey medium grained sand, trace shell fragments						s	,		18,25 for 100,- refusal
	- 15	- grey medium grained sand						S			15,30 for 80mm,- refusał
	- 16 - 16.5 - 17 - 17	CLAY - grey clay				A super and and and and and and and and and and					
	17.5	Bore discontinued at 17.5m (limit of investigation)									

RIG: Scout **DRILLER:** Driver TYPE OF BORING: Spiral flight auger to 5.5m; Rotary to 17.5m WATER OBSERVATIONS: Free groundwater observed at 4.0m **REMARKS:** E 297 326 N 6149035 (MGA)

 SAMPLING & IN SITU TESTING LEGEND

 pp
 Pocket penetrometer (kPa)

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 PID

 mm dia.)
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 V
 Shear Vane (kPa)

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 Vater seep

 Water level

 Auger sample Disturbed sample Buik sample Tube sample (x mm dia.) Water sample Core dnilling A D B U, W C Initials: Date:

LOGGED: L McKenzie

CHECKED

7 á. CASING: to 5.5m



CLIENT: Cleary Bros (Bombo) Pty Ltd PROJECT:

Proposed Northern Extension LOCATION: Gerroa Sand Quarry, Gerroa

SURFACE LEVEL: 5.8m EASTING: 280102 NORTHING: 1149688 DIP/AZIMUTH: 90°/--

BORE No: 204 PROJECT No: 37673 DATE: 21 Dec 04 SHEET 1 OF 2

		Description	Degree of Weathering	<u>.0</u>	Rock Strength	5	Fracture	Discontinuities	Sampling &		1g &	In Situ Testing
RL	(m)	of		braph Log		Vate	Spacing (m)	B - Bedding J - Joint	be	ere Sue	a ~	Test Results
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	0.6	SILTY SAND - grey brown sity sand, rootlets, damp (topsoil) SILTY SAND - very loose, orange brown silty sand, damp, rootlets							А			
	- - - -			• • •					A,S			2,2,2 N = 4
		- no rootiets			Anna Anna Anna An							
	-2	- no rootlets, wet							s			1,2,2 N = 4
	-3								s			1,2,1 N = 3
		- loose sand, no rootlets				¥			s			1,4,5 N = 9
	-5	- rootlets, wet		· · · · · · · · · · · · · · · · · · ·					s		- -	2,2,2 N = 4
	-6 6.0	SAND - dense, orange brown and grey fine to medium grained san, trace shell fragments, saturated			And And And And and And and And And And And And And And And And A				s			10,19,18 N = 37
	-7	- with some shell fragments	 State when your your your your State your your your your your State your your your your your State your your your your your State your your your your 				 Market Market Market Market Market Market Market Market Market Market Market Market Market Market Market Market Market Market Market Market Market 		S			20,25 for 80mm,- refusal
	8 8.0	GRAVELLY SAND - orange grey coarse grained sand and subrounded quartz and siltstone gravel, trace cobbles with some shells and shell fragments, saturated		0000					S			20,25 for 100mm,- refusal

RIG: Scout **DRILLER:** Driver TYPE OF BORING: Spiral flight auger to 5.5m; Rotary to 15.5m WATER OBSERVATIONS: Free groundwater observed at 4.0m **REMARKS:** E 297210 N 6148968 (MGA)

SAMPLING & IN SITU TESTING LEGEND pp Pocket penetrometer (kPa) PID Photo ionisation detector S Standard penetration test mm dia.) PL Point load strength Is(50) MPa V Shear Vane (kPa) D Water seep ¥ Water level CHECKED SAMP Auger sample Disturbed sample Bulk sample Tube sample (x mm dia.) Water sample ADBU,WC Initials: Min **Douglas Partners** 7 Core drilling Date σí Geotechnics · Environment · Groundwater



LOGGED: L McKenzie

SURFACE LEVEL: 5.8m EASTING: NORTHING:

1149688 DIP/AZIMUTH: 90°/--

280102

BORE No: 204 PROJECT No: 37673 DATE: 21 Dec 04 SHEET 2 OF 2

	_	Description	Degree of Weathering	. <u>c</u>	Rock Strength	Fracture	Discontinuities	Sa	mplin	ıg & I	n Situ Testing
RL	Depth (m)	of		Log		Spacing (m)	B - Bedding J - Joint	ed	ere %	a.°	Test Results
		Strata	H M M M M M M M M M M M M M M M M M M M	ن	Ex Low Low Very Ex High	0.01 0.10 1.00	S - Shear D - Drill Break	Ţ	ပိမ္မိ	<u>к</u> ,	Comments
	-	GRAVELLY SAND - orange grey coarse grained sand and subrounded quartz and siltstone gravel, trace cobbles with some shells and shell fragments, saturated (continued)		0000							
	- 10 10.0	SAND - medium dense, dark brown grey coarse grained sand, with some gravel, shell fragments, trace organics, cobbles, saturated	ana ana ang ang ang ang ang ang ang ang					s			11,10,9 N = 19
	- - 11 - - - -	- dense sand	to and the two of the two of the two of the two of the two of the two of the two of the two of the two of the two of the two of the two of the two of the two of the two of the two of the two of two of the two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of two of					s			12,19,26 N = 45
	- 12	- dense sand, slightly gravelly, no organics						s			11,19,30 N = 49
	-13	- grey, trace gravel, no shell fragments or organics						S			14,23,25 for 100mm
	- 14		Anna Anna Anna Anna Anna Anna Anna Anna								Totusar
	14.6	CLAY - soft, grey clay, damp		//				s			
	- 15 - 15 - 15.45	-						s			
		Bore discontinued at 15.45m (limit of investigation)				and a second a					· · · ·
	- 16					The second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second secon					
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RIG: Scout **DRILLER:** Driver TYPE OF BORING: Spiral flight auger to 5.5m; Rotary to 15.5m WATER OBSERVATIONS: Free groundwater observed at 4.0m REMARKS: E 297210 N 6148968 (MGA)

CLIENT:

PROJECT:

Cleary Bros (Bombo) Pty Ltd

Proposed Northern Extension

LOCATION: Gerroa Sand Quarry, Gerroa

SAMPLING & IN SITU TESTING LEGEND pp Pocket penetrometer (kPa) le PID Photo ionisation detector Standard penetration test mm dia.) PL Point load strength Is(50) MPa V Shear Vane (kPa) D Water seep ¥ Water level SAMP: Auger sample Disturbed sample Bulk sample Tube sample (x mm dia.) Water sample Core drilling CHECKED AD8U, ₩U Initials: Shu 2/01 Date:



LOGGED: L McKenzie



Douglas Partners

Geotechnics · Environment · Groundwater

DESCRIPTIVE GEOLOGICAL LOG

Drill Hole Reference:CB#201Collar Height:0.6 m AHDWater Table:0.1 m

Sample Interval (m)	Description
0.0 - 2.5	SAND, fawn, fine grained, well sorted, subangular-subrounded, quartzose sand. Light to medium iron stain on 30% of quartz grains. Trace lithics, no shell.
2.5 - 5.4	SAND, grey, very fine-medium grained (some coarse grained), subangular- subrounded, moderately sorted, quartzose sand. 5% shell occurring as fragments of medium-coarse grained marine shells. Occasional quartz and lithic pebbles, and thin interbedded gravel layers. 10% lithics. Many quartz grains have black (light to moderate) coating.
5.4 - 6.3	SAND, grey-brown, very fine-coarse grained (some very coarse grained), subangular-rounded, poorly sorted, quartzose sand. Trace shell, 5% lithics. Light organic (brown) coating on 5% of quartz grains.
6.3 - 6.5	CLAY, dark grey/black, plastic clay.

End of hole: - 5.9 m AHD

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DESCRIPTIVE GEOLOGICAL LOG

Drill Hole Reference: CB#204 Collar Height: 0.87 m AHD

4577 1011 1

Water Table: At or near surface

Sample Interval (m)	Description
0.0 - 0.6	SAND, light grey, fine grained, well sorted, subangular-subrounded quartzose sand. No shell, trace lithics.
 0.6 - 1.4	SAND, grey, fine-medium grained, well sorted, subangular-subrounded quartzose sand. Slightly muddy with occasional coarse grained quartz grains. <5% lithics, no shell.
1.4 - 5.3	SAND, grey, medium-coarse grained, poorly sorted, subangular-subrounded (coarse grained-very coarse grained is subrounded-rounded) quartzose sand. Clean becoming very slightly muddy with depth. Interbedded very coarse grained sand and pebble layers. 5% lithics. Trace shell occurring as medium-very coarse grained abraded fragments of marine shell.
5.3 - 5.5	CLAY, dark grey/black plastic clay.

End of hole: - 4.63 m AHD

DESCRIPTIVE GEOLOGICAL LOG

Drill Hole Reference:CB#206Collar Height:4.78 m AHDWater Table:4.5 m

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	Sample Interval (m)	Description
	0.0 - 5.5	SAND, fawn brown becoming fawn below 1.4m, fine grained, well sorted, subangular-subrounded quartzose sand. Light to moderate iron staining on 20-30% of quartzose grains. Trace lithics, no shell.
111	5.5 - 7.9	SAND, grey fawn, fine-medium grained, well sorted, subangular-subrounded quartzose sand with scattered pebbles and gravel. 5%-10% shell as medium- very coarse grained and larger abraded and whole marine shell (bankivia etc). Trace lithics. Light to heavy iron staining on 30-40% of quartz grains.
	7.9 - 9.7	SAND, fawn to light brown fawn, fine- coarse grained, poorly sorted, subangular-subrounded quartzose sand with pebbles. Trace lithics. 5% shell as medium-very coarse grained abraded fragments and larger whole marine shell. Light to moderate iron staining on 10% of grains.
	9.7 - 12.0	SAND, grey brown to brown grey, fine- medium grained, well sorted, subangular-subrounded quartzose sand. Becomes finer with depth. No shell, trace lithics. Interbedded fine and medium grained sands.
-	12.0 - 13.8	SAND, Brown grey, fine-coarse grained, moderately sorted, subangular- subrounded, guartoze sand. No shell, trace lithics.
	13.8 - 14.0	CLAY, dark grey/black plastic clay.

End of hole: - 9.22 m AHD

4

- 32 -

5.5-7 m Sand - light grey, coarse grained, high shell

		content. Becomes darker in colour, coarser
		grained and higher in shell with depth.
	7-8.5 m	Sand - medium grained, highly shelly numerous small whole shells.
	8 5-10 m	Sand - very coarse grained, rounded grains, low
	0.0 10 m	shell content.
	10-11.5 m	Clay - grey: plastic, moist.
	•	
	<u>Hole SM4</u>	
	0-1.5 m	Sand - cream, very fine grained sand.
	1.5-3 m	Sand - cream high shell content, fine grained.
•	3-4.5 m	Sand - coarse grained, higher shell content
		cream to yellow sand.
	4.5-6 m	Sand - medium to coarse grained, small whole and
		fragmented shells, grey, slightly clayey.
	6-9 m	Sand - very coarse grained, gritty, composed of
		shell and quartz grains, difficult to penetrate.
	<u>Hole SM5</u>	
	0-1.5 m	Sand - very fine grained, cream to yellow,
		free of shell.
	1.5-3 m	Sand - cream, becoming "cleaner" with depth.
	3-4.5 m	Sand - grey, fine grained, becomes more shelly
		with depth.
	4.5-6 m	Sand - grey, medium to coarse grained high shell
		content in the form of whole and fragmented shell.
		Scattered quartz pebbles.
	6-7.5 m	Sand - coarse sand intermixed with fine sand,
		shell throughout.
	7.5-9 m	Sand - clean, coarse grained, becoming more
		gritty towards base of hole. High shell
		content (whole and fragmented shell). Scattered
		quartz pebbles (up to 5 cm long).
	Hole SM6	
	0-1.5 m	Sand - orange, very fine grained.
	1.5-3 m	Sand cream-orange very fine grained, no shell
		or clay.

3-4.5 m Sand - cream to yellow. fine to medium grained, little shell, moist.
4.5-6 m Sand - medium to coarse grained, low shell in the form of scattered fragments. grey.
6-9 m Sand - very coarse grained, becoming more gritty

with depth. Shell content increasing with depth. Poorly sorted large scattered quartz pebbles. Mud - clay at base of hole.

Hole SM7

0-1.5 m	Sand - orange, medium to coarse grained.
1.5-3 m	Sand - medium to coarse grained, yellow - orange.
3.0-4.5 m	Sand - coarse and gritty, clean rounded grains.
4.5-6 m	Sand - white to cream, very coarse grained,
	high charcoal content. Little shell.
675m	Sand - medium to coarse grained, poorly sorted.
7.5-9 m	Clay - plastic moist, black - grey. No shell,
	slightly sandy.

Hole SM8

	· · · · · · · · · · · · · · · · · · ·
0-1.5 m	Sand - cream to yellow, fine grained, no shell.
1.5-3 m	Sand - medium grained, yellow - orange.
3-4.5 m	Sand - medium grained, grey, minor shell
	content (fragmented).
4.5-6 m	Sand - grey, medium grained, poorly sorted,
•	uneven grainsize, fragmented shells throughout.
6-7.5 m	Sand - very high shell content (small whole
	shells and fragments) coarse grained poorly
	sorted, grey.
7.5-9 m	Sand - coarser grained shelly as above.
9-10.5 m	Sand - very coarse grained, fine gravel, lithic
	clasts, pebbles up to 5 cm in length composed of
	sandstone, chert,quartz. All clasts are well
	rounded, mixed with small shells and shell
	fragments. Occasional clasts up to 10 cm in
· · · ·	length. Some charcoal.

APPENDIX B Results of Current Laboratory Physical Testing



Unit 1, 1 Luso Drive Unanderra NSW 2526 (02) 4271 1836 Phone Fax: (02) 4271 1897 wollongong@douglaspartners.com.au

RESULTS OF PARTICLE SIZE DISTRIBUTION TEST

Client :	CLEARY BROS (BOMBO) PTY LTD	Project No. : Poport No. :	37673
Project :	Proposed Northern Extension	Report Date :	11/01/2005
Location: Test Location:	Gerroa Sand Quarry, Gerroa Bore 201	Date Sampled: Date of Test:	20/12/2004 6/01/2005
Depth / Layer :	1.0m	Page:	1 of 1

AUSTRALIAN STANDARD SIEVE APERTURES



Brown sand

Test Method(s):

AS 1289.1.2.1-1998, AS 1289.1.1-2001

AS 1289.3.6.1-1995, 3.6.2-1995, 3.6.3-1995

Method of Dispersion:

Sampling Method(s):

Remarks:



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Tested:	PB
Checked:	DE

David Evans Laboratory Manager



Unit 1, 1 Luso Drive Unanderra NSW 2526 Phone (02) 4271 1836 Fax: (02) 4271 1897 wollongong@douglaspartners.com.au

RESULTS OF PARTICLE SIZE DISTRIBUTION TEST

Client :	CLEARY BROS (BOMBO) PTY LTD	Project No. :	37673
Project :	Proposed Northern Extension	Report No. : Report Date :	UL05-004B 11/01/2005
Location : Test Location :	Gerroa Sand Quarry, Gerroa Bore 201	Date Sampled:	20/12/2004
Depth / Layer :	2.0m	Page:	1 of 1

AUSTRALIAN STANDARD SIEVE APERTURES



Sampling Method(s): AS 1289.1.2.1-1998, AS 1289.1.1-2001

Method of Dispersion:

Remarks:



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Checked:	DE

David Evans Laboratory Manager



AUSTRALIAN STANDARD SIEVE APERTURES

Unit 1, 1 Luso Drive Unanderra NSW 2526 Phone (02) 4271 1836 Fax: (02) 4271 1897 wollongong@douglaspartners.com.au

RESULTS OF PARTICLE SIZE DISTRIBUTION TEST

Client :	CLEARY BROS (BOMBO) PTY LTD	Project No. :	37673
Project :	Proposed Northern Extension	Report Date :	11/01/2005
Location : Test Location : Depth / Laver :	Gerroa Sand Quarry, Gerroa Bore 201 8 0m	Date Sampled: Date of Test: Page:	20/12/2004 6/01/2005 1. of 1
Depth / Layer :	8.0m	Page:	1 of 1



CLAY FRACTION	SILT	FRACTIC)N	SANE	D FRACTI	ON	GRAV	EL FRACI	FION	COBBLES
	Fine	Medium	Coarse	Fine	Medium	Coarse	Fine	Medium	Coarse	
0.0	0.0	106 O.	02 02	.6	1 I 1.2 0.	6 2	6 .0	.0 2	1 20 6	50

Description:

Brown sand

Test Method(s): Sampling Method(s):

AS 1289.1.2.1-1998, AS 1289.1.1-2001

AS 1289.3.6.1-1995, 3.6.2-1995, 3.6.3-1995

Method of Dispersion:

Remarks:



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Tested:	PB
Checked:	DE

David Evans Laboratory Manager



Unit 1, 1 Luso Drive Unanderra NSW 2526 Phone (02) 4271 1836 Fax: (02) 4271 1897 wollongong@douglaspartners.com.au

RESULTS OF PARTICLE SIZE DISTRIBUTION TEST

Client :	CLEARY BROS (BOMBO) PTY LTD	Project No. :	37673
Project :	Proposed Northern Extension	Report Date :	0L05-004D 11/01/2005
Location: Test Location: Depth / Layer:	Gerroa Sand Quarry, Gerroa Bore 201 11.0m	Date Sampled: Date of Test: Page:	20/12/2004 6/01/2005 1 of 1

AUSTRALIAN STANDARD SIEVE APERTURES



Description:	
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Test Method(s):

Grey sand

0.002

AS 1289.3.6.1-1995, 3.6.2-1995, 3.6.3-1995

Sampling Method(s):

AS 1289.1.2.1-1998, AS 1289.1.1-2001

0.6

Method of Dispersion:

Remarks:



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Tested:	PB
Checked:	DE

David Evans Laboratory Manager

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RESULTS OF PARTICLE SIZE DISTRIBUTION TEST

Client :	CLEARY BROS (BOMBO) PTY LTD	Project No. :	37673
Project :	Proposed Northern Extension	Report No. : Report Date :	UL05-004E 11/01/2005
Location : Test Location : Depth / Layer :	Gerroa Sand Quarry, Gerroa Bore 202 1.5m	Date Sampled: Date of Test: Page:	20/12/2004 6/01/2005 1 of 1

AUSTRALIAN STANDARD SIEVE APERTURES



Description:

Test Method(s):

Yellow brown sand

AS 1289.3.6.1-1995, 3.6.2-1995, 3.6.3-1995

Sampling Method(s):

AS 1289.1.2.1-1998, AS 1289.1.1-2001

Method of Dispersion:

Remarks:



Form R004 Rev3 Feb 2004

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Tested;	PB
Checked:	DE

David Evans Laboratory Manager



Unit 1, 1 Luso Drive Unanderra NSW 2526 Phone (02) 4271 1836 Fax: (02) 4271 1897 wollongong@douglaspartners.com.au

RESULTS OF PARTICLE SIZE DISTRIBUTION TEST

Client :	CLEARY BROS (BOMBO) PTY LTD	Project No. :	37673
Project :	Proposed Northern Extension	Report No. : Report Date :	UL05-004F 11/01/2005
Location : Test Location :	Gerroa Sand Quarry, Gerroa Bore 202	Date Sampled: Date of Test:	20/12/2004
Depth / Layer :	3.0m	Page:	1 of 1

AUSTRALIAN STANDARD SIEVE APERTURES



Sampling Method(s): AS 1289.1.2.1-1998, AS 1289.1.1-2001

Method of Dispersion:

Remarks:



⁻orm R004 Rev3 Feb 200[,]

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RESULTS OF PARTICLE SIZE DISTRIBUTION TEST

Client :	CLEARY BROS (BOMBO) PTY LTD	Project No. : Report No. :	37673 UL05-004G
Project :	Proposed Northern Extension	Report Date :	11/01/2005
Location :	Gerroa Sand Quarry, Gerroa	Date Sampled:	20/12/2004
Test Location :	Bore 202	Date of Test:	6/01/2005
Depth / Layer :	5.0m	Page:	1 of 1

AUSTRALIAN STANDARD SIEVE APERTURES



	CLAY FRACTION	SILT FRACTION		SAND FRACTION		GRAVEL FRACTION			COBBLES		
1		Fine	Medium	Coarse	Fine	Medium	Coarse	Fine	Medium	Coarse	
	0.0	0.0	1 006 0.	02 02	.6	.2 0	.6 2	6 .0	.0 2	1 :0 6	0

Description:

Brown sand

Test Method(s):

AS 1289.3.6.1-1995, 3.6.2-1995, 3.6.3-1995

Sampling Method(s):

AS 1289.1.2.1-1998, AS 1289.1.1-2001

Method of Dispersion:

Remarks:



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RESULTS OF PARTICLE SIZE DISTRIBUTION TEST

Client :	CLEARY BROS (BOMBO) PTY LTD	Project No. :	37673
Project :	Proposed Northern Extension	Report No. : Report Date :	UL05-004H 11/01/2005
Location : Test Location : Depth / Layer :	Gerroa Sand Quarry, Gerroa Bore 202 7.0m	Date Sampled: Date of Test: Page:	20/12/2004 6/01/2005 1 of 1

AUSTRALIAN STANDARD SIEVE APERTURES



Test Method(s):

Sampling Method(s):

AS 1289.1.2.1-1998, AS 1289.1.1-2001

AS 1289.3.6.1-1995, 3.6.2-1995, 3.6.3-1995

Method of Dispersion:

Remarks:



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RESULTS OF PARTICLE SIZE DISTRIBUTION TEST

Client :	CLEARY BROS (BOMBO) PTY LTD	Project No. :	37673
Project :	Proposed Northern Extension	Report Date :	11/01/2005
Location : Test Location : Depth / Layer :	Gerroa Sand Quarry, Gerroa Bore 202 10.0m	Date Sampled: Date of Test: Page:	20/12/2004 6/01/2005 1 of 1

AUSTRALIAN STANDARD SIEVE APERTURES



Description:

Dark grey sand

Test Method(s):

Sampling Method(s):

AS 1289.1.2.1-1998, AS 1289.1.1-2001

AS 1289.3.6.1-1995, 3.6.2-1995, 3.6.3-1995

Method of Dispersion:

Remarks:



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RESULTS OF PARTICLE SIZE DISTRIBUTION TEST

Client :	CLEARY BROS (BOMBO) PTY LTD	Project No. : Report No. :	37673 LIL 05-004 L
Project :	Proposed Northern Extension	Report Date :	11/01/2005
Location : Test Location : Depth / Layer :	Gerroa Sand Quarry, Gerroa Bore 203 2.0m	Date Sampled: Date of Test: Page:	20/12/2004 6/01/2005 1 of 1

AUSTRALIAN STANDARD SIEVE APERTURES



Sampling Method(s):

AS 1289.1.2.1-1998, AS 1289.1.1-2001

Method of Dispersion:

Remarks:



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RESULTS OF PARTICLE SIZE DISTRIBUTION TEST

Client :	CLEARY BROS (BOMBO) PTY LTD	Project No. :	37673
Project :	Proposed Northern Extension	Report Date :	0L05-004K 11/01/2005
Location : Test Location : Depth / Layer :	Gerroa Sand Quarry, Gerroa Bore 203 6.0m	Date Sampled: Date of Test: Page:	20/12/2004 6/01/2005 1 of 1

AUSTRALIAN STANDARD SIEVE APERTURES



Description:

Yellow brown sand

Test Method(s):

AS 1289.1.2.1-1998, AS 1289.1.1-2001

AS 1289.3.6.1-1995, 3.6.2-1995, 3.6.3-1995

Sampling Method(s): Method of Dispersion:

Remarks:



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RESULTS OF PARTICLE SIZE DISTRIBUTION TEST

Client :	CLEARY BROS (BOMBO) PTY LTD	Project No. : Report No. :	37673
Project :	Proposed Northern Extension	Report Date :	11/01/2005
Location : Test Location : Depth / Layer :	Gerroa Sand Quarry, Gerroa Bore 203 9.0m	Date Sampled: Date of Test: Page:	20/12/2004 6/01/2005 1 of 1

AUSTRALIAN STANDARD SIEVE APERTURES



Description:	
--------------	--

Brown gravelly sand

Test Method(s):

own gravelly sand

Sampling Method(s):

AS 1289.1.2.1-1998, AS 1289.1.1-2001

AS 1289.3.6.1-1995, 3.6.2-1995, 3.6.3-1995

Method of Dispersion:

Remarks:



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RESULTS OF PARTICLE SIZE DISTRIBUTION TEST

Client :	CLEARY BROS (BOMBO) PTY LTD	Project No. : Report No. :	37673 UL05-004M
Project :	Proposed Northern Extension	Report Date :	11/01/2005
Location : Test Location : Depth / Layer :	Gerroa Sand Quarry, Gerroa Bore 203 12.0m	Date Sampled: Date of Test: Page:	20/12/2004 6/01/2005 1 of 1

AUSTRALIAN STANDARD SIEVE APERTURES



Sampling Method(s): AS 1289.1.2.1-1998, AS 1289.1.1-2001

Method of Dispersion:

Remarks:



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Checked:	DE

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RESULTS OF PARTICLE SIZE DISTRIBUTION TEST

Client :	CLEARY BROS (BOMBO) PTY LTD	Project No. :	37673
Project :	Proposed Northern Extension	Report No. : Report Date :	0L05-004N 11/01/2005
Location : Test Location : Depth / Layer :	Gerroa Sand Quarry, Gerroa Bore 204 0.5m	Date Sampled: Date of Test: Page:	20/12/2004 6/01/2005 1 of 1

AUSTRALIAN STANDARD SIEVE APERTURES



Test Method(s):

AS 1289.3.6.1-1995, 3.6.2-1995, 3.6.3-1995

Sampling Method(s):

AS 1289.1.2.1-1998, AS 1289.1.1-2001

Method of Dispersion:

Remarks:



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RESULTS OF PARTICLE SIZE DISTRIBUTION TEST

Client :	CLEARY BROS (BOMBO) PTY LTD	Project No. : Report No. :	37673 UL05-004O
Project :	Proposed Northern Extension	Report Date :	11/01/2005
Location : Test Location : Depth / Layer :	Gerroa Sand Quarry, Gerroa Bore 204 2.0m	Date Sampled: Date of Test: Page:	20/12/2004 6/01/2005 1 of 1

AUSTRALIAN STANDARD SIEVE APERTURES



Description:

Yellow brown sand

Test Method(s):

CHOW DIOWIT SAILU

Sampling Method(s):

AS 1289.1.2.1-1998, AS 1289.1.1-2001

AS 1289.3.6.1-1995, 3.6.2-1995, 3.6.3-1995

Method of Dispersion:

Remarks:



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Checked:	DĘ

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RESULTS OF PARTICLE SIZE DISTRIBUTION TEST

Client :	CLEARY BROS (BOMBO) PTY LTD	Project No. :	37673
Project :	Proposed Northern Extension	Report No. : Report Date :	UL05-004P 11/01/2005
Location : Test Location : Depth / Layer :	Gerroa Sand Quarry, Gerroa Bore 204 4.0m	Date Sampled: Date of Test: Page:	20/12/2004 6/01/2005 1 of 1

AUSTRALIAN STANDARD SIEVE APERTURES



Sampling Method(s):

AS 1289.1.2.1-1998, AS 1289.1.1-2001

Method of Dispersion:

Remarks:



⁻orm R004 Rev3 Feb 2004

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David Evans Laboratory Manager



Unit 1, 1 Luso Drive Unanderra NSW 2526 Phone (02) 4271 1836 Fax: (02) 4271 1897 wollongong@douglaspartners.com.au

RESULTS OF PARTICLE SIZE DISTRIBUTION TEST

Client :	CLEARY BROS (BOMBO) PTY LTD	Project No. :	37673
Project :	Proposed Northern Extension	Report No. : Report Date :	UL05-004Q 11/01/2005
Location : Test Location : Depth / Layer :	Gerroa Sand Quarry, Gerroa Bore 204 7.0m	Date Sampled: Date of Test: Page:	20/12/2004 6/01/2005 1 of 1

AUSTRALIAN STANDARD SIEVE APERTURES



CLAT FRACTION	SIL		л л	SAN	JERAUTI	UN	GRAV	EL FRAC	HON	COBBLES
	Fine	Medium	Coarse	Fine	Medium	Coarse	Fine	Medium	Coarse	
0.0	0.0 002)06 0.	02 02	.e	0.2 0	.6 2	 .0	3.0 2	1 206	60

Description:

Brown gravelly sand

Test Method(s):

AS 1289.3.6.1-1995, 3.6.2-1995, 3.6.3-1995

Sampling Method(s):

AS 1289.1.2.1-1998, AS 1289.1.1-2001

Method of Dispersion:

Remarks:



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Tested:	PB
Checked:	DE

David Evans Laboratory Manager



Unit 1, 1 Luso Drive Unanderra NSW 2526 Phone (02) 4271 1836 Fax: (02) 4271 1897 wollongong@douglaspartners.com.au

RESULTS OF PARTICLE SIZE DISTRIBUTION TEST

Client :	CLEARY BROS (BOMBO) PTY LTD	Project No. :	37673
Project :	Proposed Northern Extension	Report No. : Report Date :	UL05-004R 11/01/2005
Location : Test Location : Depth / Layer :	Gerroa Sand Quarry, Gerroa Bore 204 12.0m	Date Sampled: Date of Test: Page:	20/12/2004 6/01/2005 1 of 1

AUSTRALIAN STANDARD SIEVE APERTURES



Description:

Dark brown sand

Test Method(s):

Sampling Method(s):

AS 1289.1.2.1-1998, AS 1289.1.1-2001

AS 1289.3.6.1-1995, 3.6.2-1995, 3.6.3-1995

Method of Dispersion:

Remarks:



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Checked:	DE

David Evans Laboratory Manager



Douglas Partners Pty Ltd ABN 75 053 980 117 Box 324 Hunter Region Mail Centre NSW 2310 Australia 15 Callistemon Close Warabrook NSW 2304

Phone (02) 4960 9600 Fax: (02) 4960 9601 newcastle@douglaspartners.com.au

RESULTS OF PARTICLE DENSITY & WATER ABSORPTION (FINE)

Client: Project:	Cleary Bros (Bo Proposed Northe	ombo) Pty ern Extensi	Ltd on	Project No: Report No: Report Date:	37673 N05-018 3/2/2005
Location:	Gerroa Sand Qu	arry, Gerro	ba	Date Sampled: Date of Test: Page:	: 20-23/12/2004 20/1/2005 1 of 1
Sample Detai	ls		Bore 202 - 1.0m (Sand)	Bore 202 - 6.0m (Sand)	Bore 203 - 3.0m (Sand)
Apparent Par	ticle Density	t/m³	2.37	2.34	2.35
Particle Dens	ity on Dry Basis	t/m³	2.32	2.32	2.33
Particle Dens Surface Dry I	ity on Saturated Basis	t/m ³	2.34	2.33	2.34
Water Absor	otion	%	1.0	0.4	0.6

 Test Method(s):
 AS1141.5.1 - 2000

 Sampling Method(s):
 AS1141.3.1 - 1996

 Remarks:
 AS1141.3.1 - 1996



NATA Accredited Laboratory Number: 1670 NATA endorsed test report. This document shall not be reproduced, except in full. Approved Signatory:

fller

Tested: AW/JP Checked: DM D Millard Laboratory Manager



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 Phone
 (02) 4960 9600

 Fax:
 (02) 4960 9601

 newcastle@douglaspartners.com.au

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RESULTS OF PARTICLE DENSITY & WATER ABSORPTION (FINE)

Client: Project:	Cleary Bros (Bon Proposed Norther	n bo) Pty n Extensi	Project No: Report No: Report Date:	37673 N05-018a 3/2/2005	
Location:	Gerroa Sand Qua	rry, Gerro	Date Sampled Date of Test: Page:	: 20-23/12/2004 20/1/2005 1 of 1	
Sample Detai	S		Bore 203 - 11.0m (Sand)	Bore 204 3.0-5.0m (Sand)	
Apparent Par	icle Density	t/m ³	2.37	2.38	
Particle Dens	ity on Dry Basis	t/m³	2.32	2.36	
Particle Dens Surface Dry E	ity on Saturated Basis	t/m ³	2.34	2.37	
Water Absorp	otion	%	1.0	0.3	

 Test Method(s):
 AS1141.5.1 - 2000

 Sampling Method(s):
 AS1141.3.1 - 1996

 Remarks:
 AS1141.3.1 - 1996

NATA Accredited Laboratory Number: 1670 NATA endorsed test report. This document shall not be reproduced, except in full. Approved Signatory:

AW/JP

Tested:

Checked: DM

D Millard Laboratory Manager



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15 Callistemon Close Warabrook NSW 2304

(02) 4960 9600 Phone Fax: (02) 4960 9601 newcastle@douglaspartners.com.au

RESULTS OF ORGANIC IMPURITIES OTHER THAN SUGAR

Client: Project:	Cleary Bros (Bombo) Pty Ltd Proposed Northern Extension			Project No: Report No: Report Date:	37673 N05-018b 3/2/2005
Location:	Gerroa S	Sand Quarry, Gerroa	Date Sampled: Date of Test: Page:	20-23/12/2004 20/1/2005 1 of 1	
Sample D	etails:	Bore 201 (1.5m)	Bore 201 (6.0m)	Bore 204 (8.0m)	
Result:		FAIL	PASS	PASS	

Test Method(s): AS 1141.34 - 1997 Sampling Method(s): AS1141.3.1-1996 Remarks:

NATA Accredited Laboratory Number: 1670 NATA endorsed test report. This document shall not be reproduced, except in full. **Approved Signatory:**

Tested:

AW Checked: DM

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D Millard Laboratory Manager

APPENDIX C Results of Previous Laboratory Physical Testing



test results

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Consulting Engineers in the geotechnical sciences Incorporated in NSW

Laboratory: Wollongong

client: Cleary Bros. (Bombo) I project: Gerroa Sand Resource location: Gerroa	'ty Ltd date: 26-7-90 job no: SC568/1
test procedure: AS 1289 C6.1	(1977)
	DATE RECEIVED: 20-7-90
SAMPLE NUMBER	TEST RESULT
CB201 3.0 - 5.0m	
6.7mm Sieve 4.75mm Sieve 2.36mm Sieve 1.18mm Sieve 600 µm Sieve 425 µm Sieve 300 µm Sieve 150 µm Sieve 75 µm Sieve SHELL ANALYSIS (Sieve Freetien)	100 98 97 95 77 54 38 12 6 % SHELL*
-4.75mm to +1.18mm	15.8
-1.18mm to +75 μm	8.2
OMYRIGHT @ COFFEY PARTHERS INTERNATIO	 NOTE: 1. Shell content determined by non-standard method. For test method refer to covering letter of results. * % Shell may include other soil particles susceptible to HCL acid attack. - Sample to small for shell analysis. *

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test results

Consulting Engineers in the geotechnical sciences incorporated in NSW

Laboratory: Wollongong

client: project: location:	Cleary Bros. (Bombo) Pty Ltd Gerroa Sand Resource Gerroa		date: 26-7-90	job na:	SC568/1
test procedu	re: AS 1289 C6.1 (1977)	 			

DATE RECEIVED: 20-7-90 SAMPLE NUMBER TEST RESULT CB201 4.9 - 5.4m 26.5mm Sieve 100 19.0mm Sieve 98 13.2mm Sieve 96 9.5mm Sieve 95 6.7mm Sieve 94 4.75mm Sieve 94 2.36mm Sieve 92 1.18mm Sieve 88 600 µm Sieve 76 425 µm Sieve 59 300 µm Sieve 43 150 µm Sieve 13 75 µm Sieve 5 SHELL ANALYSIS % SHELL* (Sieve Fraction) -4.75mm to +1.18mm 10.2 6.2 -1.18mm to +75 µm NOTE: Shell content determined by non-standard method. 1.

- For test method refer to covering letter of results.
- * % Shell may include other soil particles susceptible to HCL acid attack.
- Sample to small for shell analysis.



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test results

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Consulting Engineers in the geotechnical sciences Incorporated in NSW

Laboratory: Wollongong

client: Cleary Bros. (Bombo) Pty project: Gerroa Sand Resource location: Gerroa	date: 26-7-90 job no: SC568/1
test procedure: AS 1289 C6.1	(1977)
	DATE RECEIVED: 20-7-90
SAMPLE NUMBER	TEST RESULT
CB2O4 O - 1.Om	
1.18mm Sieve 600 µm Sieve 425 µm Sieve 300 µm Sieve 150 µm Sieve 75 µm Sieve	100 98 93 86 17 12
SHELL ANALYSIS (Sieve Fraction) -4.75mm to +1.18mm -1.18mm to +75 µm	<pre>% SHELL* 82.1 7.5 * NOTE: 1. Shell content determined by non-standard method. For test method refer to covering letter of results. * % Shell may include other soil particles susceptible to HCL acid attack. - Sample to small for shell analysis.</pre>



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Coffey Partners International Pty Ltd Incorporated in NSW

particle size distribution



borehole no CB204



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Authorised Signature P.R. Sherley



test results

Consulting Engineers in the geotechnical sciences Incorporated in NSW

Laboratory: Wollongong

client: Cleary Bros. (Bombo) Ply project: Gerroa Sand Resource location: Gerroa	- Ltd date: 26-7-90 job no: SC568/1
test procedure: AS 1289 C6.1	(1977)
	DATE RECEIVED: 20-7-90
SAMPLE NUMBER	TEST RESULT
CB204 2.0 - 5.0m	
6.7mm Sieve 4.75mm Sieve 2.36mm Sieve 1.18mm Sieve 600 µm Sieve 425 µm Sieve 300 µm Sieve 150 µm Sieve 75 µm Sieve	100 98 97 91 60 34 19 6 4
SHELL ANALYSIS (Sieve Fraction)	% SHELL*
-4.75mm to +1.18mm	1.0
-1.18mm to +75 um	2.6
	NOTE:
HT © COFFEY PATTNERS	 Shell content determined by non-standard method. For test method refer to covering letter of results. * % Shell may include other soil particles susceptible to HCL acid attack. Sample to small for shell analysis.
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Coffey Partners International Pty Ltd Incorporated in NSW

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D.R. Shaley, Authonsed Signature



test results

Consulting Engineers in the geotechnical sciences Incorporated in NSW

Laboratory: Wollongong

Cleary Bros. (Bombo) Pty Ltd client: Gerroa Sand Resource project: Gerroa location: date: 26-7-90 job no: SC568/1 test procedure: AS 1289 C6.1 (1977) DATE RECEIVED: 20-7-90 SAMPLE NUMBER TEST RESULT CB206 6.0 - 8.0m 6.7mm Sieve 100 4.75mm Sieve 96 2.36mm Sieve 92 -1.18mm Sieve 90 600 µm Sieve 85 76 425 µm Sieve 300 um Sieve 63 150 jum Sieve 19 75 µm Sieve 10 % SHELL* SHELL ANALYSIS (Sieve Fraction) -4.75mm to +1.18mm 34.9 20.3 -1.18mm to +75 um NOTE: 1. Shell content determined by non-standard method. For test method refer to covering letter of results. × % Shell may include other soil particles susceptible to HCL acid attack. Sample to small for shell analysis.



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Consulting Engineers in the geotachnical sciences incorporated in NSW

test results

Laboratory: Wollongong

client: Cleary Bros. (Bombo) P project: Gerroa Sand Resource location: Gerroa	ty Ltd date: 26–7–90 job no: SC568/1
test procedure: AS 1289 C6.1	(1977)
	DATE RECEIVED: 20-7-90
SAMPLE NUMBER	TEST RESULT
CB206 8.0 - 10.0m	
9.5mm Sieve 6.7mm Sieve 4.75mm Sieve 2.36mm Sieve 1.18mm Sieve 600 μm Sieve 425 μm Sieve 300 μm Sieve 150 μm Sieve 75 μm Sieve	100 99 97 94 88 73 58 44 10 6
SHELL ANALYSIS (Sieve Fractions)	% SHELL*
-4.75mm to +1.18mm	20.3
= −1.18mm to ÷75 µm	6.3
LENIATION	NOTE:
ALTINERS IN	1. Shell content determined by non-standard method. For test method refer to covering letter of results.
COFFEY	* % Shell may include other soil particles susceptible to HCL acid attack.
OPYRIGHT ©	- Sample to small for shell analysis.

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particle size distribution



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test results

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Consulting Engineers in the geotechnical sciences locorporated in NSW

Laboratory: Wollongong

client: Cleary Bros. (Bombo) F project: Gerroa Sand Resource location: Gerroa	'ty Lid date: 26-7-90 job no: SC568/1
test procedure: AS 1289 C6.1	(1977)
	DATE RECEIVED: 20-7-90
SAMPLE NUMBER	TEST RESULT
CB206 10.0 - 13.0m	
2.36mm Sieve 1.18mm Sieve 600 µm Sieve 425 µm Sieve 300 µm Sieve 150 µm Sieve 75 µm Sieve	100 99 84 61 41 8 3
SHELL ANALYSIS (Sieve Fraction)	% SHELL*
-4./5mm to +1.18mm -1.18mm to +75wpim	41.2
OMTRIGHT © ODFFEY PATTHERS INTER	 NOTE: 1. Shell content determined by non-standard method. For test method refer to covering letter of results. * * Shell may include other soil particles susceptible to HCL acid attack. - Sample to small for shell analysis.

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particle size distribution



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TEST METHOD - SHELL CONTENT OF SAMPLE

Samples received were oven dried overnight to a constant water content and dry sieved to separate soil fractions. The sieved fractions were then spilt down to a manageable sample size for the test.

The samples were weighed and the mass recorded. Hydrochloric acid was then added to the sample until all chemical reaction was observed to have ceased. Following acid treatment the samples were washed with tap water on a 75 um sieve to dilute the hydrochloric acid.

Samples were again oven dried and allowed to cool then reweighed. The difference in mass prior to, and after the addition of hydrochloric acid was recorded as a percentage of original mass of the sample, as follows:

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 $% \text{Shell} = (\underline{M}_{\underline{n}} - \underline{M}_{\underline{B}}) \times 100$ $\underline{M}_{\underline{n}}$

% Shell = Percentage of shells in sample

 M_{A} = Mass of dried sample prior to adding acid

 $M_{\rm m}$ = Mass of dried sample after adding sample

NOTE:

1.

% Shell may also include other soil particles in the sample susceptible to acid attack. 80.00 N

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Laboratory Analyses of Sands from Gerroa Resource - Southern Extraction Area

Drill Hole No.	Depth of Hole (m)	Sample Depth (m)	% Passing 300 um	% Shell (>1.18 mm)	% Shell (<1.18 mm)	Unit				
CB#201	6.5	3 to 5	38	~	8.2	2*				
		4 <i>.</i> 9 to 5.4	43	10.2	6.2	2				
CB#202	7.5	1 to 2	53	268	0.3	2				
		2 to 3	24	8.4	0.4	2				
		4 to 6	49	-	1.0	2				
CB#203	7.0	0 to 2	66	-	0.7	2				
		3 to 6	35	1.0	0.8	2				
CB#204	5.5	0 to 1	86	-	7.5	1-2				
		2 to 5	19	1.0	2.6	2				
CB#205	10.5	0 to 6	76	-	0.5	1				
		7 to 10	41	11.0	12.1	2				
CB#206	14.0	6 to 8	63	34.9	20.3	2				
		8 to 10	44	12.0	6.3	2				
		10 to 13	41	1.0	1.6	2				
	* Disturbed									

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(No. of pages excluding cover page = 12)

Results from June 2000 Investigations

- Test Report Sheets for TP1, TP2 and TP3
- Carbonate Content

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ACN 069 211 561 Unit 1/140 Industrial Road Oak Flats NSV/ 2529 Telephone (02) 4257 4453 Facsimile (02) 4257 4463 Email_wolnetgeo@Bigpond.com

TEST REPORT SHEET			(Document No R18.0)
Client: Cleary Bros(Bombo) Pty Ltd		Job No: W2099/1	
Project: Material Testing			
Location: Gerroa Quarry		Date: 13.7.00	
SAMPLE DATA			
Sample Number:	7		
Sample Location:	TP3 1.2m depth		
Date Sampled:	28.6.00		. ,
Sampling Procedure:	AS1141.3.1		
Sample Description:	GERROA SAND		
L		1	
TEST PROCEDURE		TEST RESULTS	
AS1141.11 (1996)			
(Washed)			
* Passing 6 7mm Sieve			
% Passing 4.75mm Sieve	100		
% Passing 2.36mm Sieve	100		
% Passing 1.18mm Sieve	99 80		
S Passing 600µm Sieve	49		
% Passing 300µm Sieve	22		
% Passing 150µm Sieve	10		
% Passing 75µm Sieve	8		
AS1141.34 (1997)			
Organic Impurities			
Other Than Sugar Pass/Fail	Pass		

17.0

Comment:

AS1289 2.1.1

Moisture Content

SW - SW

1.



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9. 0

appla 13/7/60

Approved Signature



TEST REPORT SHEET			(Document No B18 m
Client: Cleary Bros(Bombo) Ply L	td	Job No: W2099/1	
Project: Material Testing			
Location: Gerroa Quarry		Date: 13.7.00	
SAMPLE DATA			· · · · · · · · · · · · · · · · · · ·
Sample Number:	8		
Sample Location:	TP3		
	4m depth		
Date Sampled:	28.6.00		. ,
Sampling Procedure:	AS1141.3.1		
Sample Description:	GERROA SAND		
# TEST PROCEDURE		TEST RESULTS	
AS1141.11 (1996) (Washed)			
% Passing 6.7mm Sieve	100		

	% Passing 6.7mm Sieve % Passing 4.75mm Sieve % Passing 2.36mm Sieve % Passing 1.18mm Sieve % Passing 600µm Sieve % Passing 425µm Sieve % Passing 300µm Sieve % Passing 150µm Sieve % Passing 75µm Sieve	100 100 99 89 - 68 40 13 7		
	AS1141.34 (1997)			
	Organic Impurities Other Than Sugar Pass/Fail	Fail		
	AS1289 2.1.1			
	Moisture Content %	19.9		
Ċc	omnent:	<u>_</u>	·····	<u> </u>



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Approved Signature 197/00



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TEST REPORT SHEET			(Document No R18 ())
Client: Cleary Bros(Bombo) Pty Ltd	······································	Job No: W2099/1	
Project: Material Testing			
Location: Gerroa Quarry		Date: 13.7.00	
SAMPLE DATA			:
Sample Number:	9		
Sample Location:	TP3 6m depth		
Date Sampied:	28.6.00		
Sampling Procedure:	AS1141.3,1		
Sample Description:	GERROA SAND		
E TEST PROCEDURE		TEST RESULTS	
AS1141.11 (1996) (Washed)			

	(washeo)			
	% Passing 6.7mm Sieve % Passing 4.75mm Sieve % Passing 2.36mm Sieve % Passing 1.18mm Sieve % Passing 600µm Sieve % Passing 425µm Sieve % Passing 300µm Sieve % Passing 150µm Sieve % Passing 75µm Sieve	100 100 99 76 . 35 . 12 5 2		
	AS1141.34 (1997)			
	Organic Impurities Other Than Sugar Pass/Fail	Fail		
	AS1289 2.1.1			
	Moisture Content %	21.3		
$\overline{\frown}$				

Comment:



THIS LABORATORY IS ACCREDITED BY THE NATIONAL ASSOCIATION OF TESTING ACTHORITICS, AUSTRALIA, THE DESIST REPORTED HERLIN, HAVE BEEN PERFORMED IN ACCORDANCE WITH ITS FERMIN OF ACCREDITATION. THIS POCUMENT SHALL NOT BE REPRODUCED ENCORF IN FULL NATIVACCREDITED No. 1313

sale 13:1:00 Approved Signature



1949 CAR

ACN 069 21:55 Unit 1/140 Industrial Road Oak Flats NSW 2525 Telephone (02) 4257 4455 Facsimile (02) 4257 4465 Email wolnetgeo@Bigpond con

TEST REPORT SHEET	·		(Document No R1)
Client: Cleary Bros(Bombo) Pty Ltd		Job No: W2099/1	
Project: Material Testing			
r rojoon materia realing			
Location: Gerroa Quarry	······	Date: 13.7.00	
SAMPLE DATA	10		
Sample Location:			
	7.5m deoth		
	r,om acom		
Date Sampled:	28.6.00		- ,
Sampling Procedure:	AS1141.3.1		
Sample Description:	GERROA SAND		-
		·	
IEST PROCEDURE	1	TEST RESULTS	
AS1141.11 (1996)			
(Washed)			
	:		
% Passing 6.7mm Sieve	100		
Passing 4.75mm Sieve	100		
% Passing 2.30mm Sleve	100		
% Passing 600µm Sieve	89		
% Passing 425µm Sieve	e 64		
% Passing 300µm Sieve	33		
% Passing 150µm Sieve	10		
hassing 75µm Sieve			
AS1141.34 (1997)			
Organic Impurities	Fait		
Uner I nan Sugar Pass/Fati	i Catr		
AS1289 2.1.1			

Comment:

Moisture Content



THIS EVADORATORY IS ACCREDITED BY THE NATIONAL ASSOCIATION OF TESTING AUTHORITIES, AUSTRALIA THE TESTIS REPORTED HERITY HAVE BEEN PERFORMED IN ACCORDANCE WITH ITS TERMS OF ACCREDITATION THIS OUCLIMENT SHALL NOT BE REPRODUCED EXCEPT IN FULL NATA ACCREDITED No. 1313

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Approved Signature.

1stries

	CARI	BONATE CONTENI (By acid digestion)	T
LAB No	SAMPLE SOURCE	DESCRIPTION	CARBONATE (%)
22873	#1	SAND	Not Detectable*
22874	#2	SAND	Not Detectable
22975	#3	SAND	Not Detectable
22876	#4	SAND	Not Detectable
22877	#5	SAND	Not Detectable
22878	#6	SAND	Not Detectable
22879	#7	SAND	Not Detectable
22880	#8	SAND	Not Detectable
22881	#9	SAND	Not Detectable
<i>₹ 1</i> 22882	#10	SAND	Not Detectable
22883	≓11	SAND	Not Detectable
Date Tested: 1 Sampled By: C	17.07.2000 LIENT	2	
LEST METHO	DS		
APHA: : below limits o	f this method: L	ess than 0.2%	
om/File CH1 : Issue	1 May 1998		
VETWORK GE	OTECHNICS	AUS 19 B	TRALIAN SUIL TESTING PTY LT ERMILL ST, ROCKDALE 2216

APPENDIX D Results of Chemical Testing
Douglas F Gootectnics · Environn	Partners nent · Groundwater				RESULTS (DF ACID SULFA	TE SCREENING TESTS	1
Client: Project:	Cleary B Propose	sros (Bombo) Pty Ltd d Northern Extension	ЦВ	Project No: bH Meter:	37673 TPS with I-7	h lonode J46/WF	280 pH/Temp. Electrode	
Project Location:	Gerroa S Gerroa	Sand Quarry	0	Calibration Buff	er:			
Sample	Depth	hF (in distilled water)	:	pH _{FOX} (oxidised in H _s	0°)	Strength of Reaction	Soil Description	
Location	Ē.	Date:23/12/04 Time:	Date:23/12/04 Time:	Date: Time:	Date: Time:	(1,2,3,4)* F **		
BH 201	A/S 0.5	6.2	5.9			-	Silty Sand	
BH 201	A/S 1.0	6.3	6.2			~	Silty Sand	
BH 201	STP 1.0	6.5	6.2			~	Silty Sand	
BH 201	A/S 1.5	6.5	6.2			~	Silty Sand	
BH 201	SPT2.0	6.6	5.8			•	Silty Sand	
BH 201	SPT 3.0	6.7	6.3			-	Sand	
BH 201	SPT 4.0	7.0	6.4			1	Sand	
BH 201	SPT5.0	7.9	7.7			-	Sand	
BH 201	SPT 6.0	7.8	7.9			~	Sand	
BH 201	SPT 7.0	7.9	8.2			~	Sand	
BH 201	SPT 8.0	8.2	7.7			~	Sand	
BH 201	SPT 9.0	8.4	7.4			-	Gravely Sand	
BH 201	SPT 10.0	8.1	5.9			~	Sand	
BH 201	SPT 11.0	8.1	5.6			-	Sand	
BH 201	SPT 12.0	8.0	4.7			-	Sand	
Legend: * 1 de 2 de	notes no or s. notes modera	light effervescence ate effervescence				Operator:	LMcK/AC	
3. a6 4. 4 ** T	notes vigorou notes "volcan	is enervescence 10" ie. very vigorous effer 11-11-11-11-11-11-11-11-11-11-11-11-11-	vescence, gas e	volution and hea	at	Date:	23 Dec. 04	
	ter reaction	number indicates a pu	poilng/rrorny re	action (organic	Ş			

CM ASS/Table 2

Rev0/March 2003

GO Douglas	Partners ment · Groundwater				RESULTS O	F ACID SULFA	TE SCREENING TESTS	1
Client: Project:	Cleary B Proposec	ros (Bombọ) Pty Ltd ď Northern Extension	ιa	roject No: h Meter:	37673 □ TPS with ⊡	lonode IJ46/WF	980 pH/Temp. Electrode	
Project Location:	Gerroa S Gerroa	and Quarry	0	alibration Buff				
Samole	Depth	pHF (in distilled water)		pH _{FOX} (oxidised in H₅	0¢)	Strength of Reaction	Soil Description	
Location	Ē	Date:22/12/04	Date:22/12/04	Date:	Date:	(1,2,3,4)*		
		Ime:	IIMe:	Ime:	IIme:	* L		
BH 201	SPT 13.0	7.9	2.5			2	Clayey Sand	
BH 202	A/S 0.5	8.3	4.8			-	Silty Sand	
BH 202	A/S 1.0	8.3	5.8			1	Silty Sand	
BH 202	SPT 1.0	7.9	5.6			1	Silty Sand	
BH 202	A/S 1.5	8.0	5.8	-		-	Silty Sand	ł
BH 202	SPT 2.0	6.8	7.0			-	Silty Sand	
BH 202	SPT 3.0	6.9	6.8			~	Silty Sand	1
BH 202	SPT 4.0	7.4	6.8			~	Silty Sand	
BH 202	SPT 5.0	8.2	7.4			-	Silty Sand	
BH 202	SPT 6.0	7.9	7.4			~	Sand	
BH 202	SPT 7.0	7.7	7.6			1	Sand	
BH 202	SPT 9.0	7.7	7.7				Silty Sand	
BH 202	SPT 10.0	7.7	5.6			~	Clayey Sand	
BH 202	SPT 11.0	7.7	4.8			1/2	Sand	
BH 202	SPT 12.0	7.7	5.9			-	Sand	
Legend: * 1d 2 d	enotes no or sl enotes modera	ight effervescence te effervescence				Operator:	LMcK/AC	

DEC LINICIVAL 23 DEC. 04

Date:

3 denotes vigorous effervescence 4 denotes "volcano" ie. very vigorous effervescence, gas evolution and heat ** F after reaction number indicates a bubbling/frothy reaction (organics

CM ASS/Table 2

Rev0/March 2003

() Douglas I Geotechnics - Environ	Partners ment - Groundwater		1		RESULTS O	F ACID SULFA	TE SCREENING TESTS	1
Client: Project:	Cleary B Propose	ros (Bombo) Pty Ltd d Northern Extension	<u> </u>	roject No: hH Meter:	37673 □ TPS with Iơ	Ionode IJ46/WF	⊃80 pH/Temp. Electrode	
Project Location:	Gerroa S Gerroa	Sand Quarry	U	alibration Buff	ar: ⊠ pH4 □ pH7 □ pH10			
Sample	Depth	pH _F (in distilled water)	-	pH _{FoX} (oxidised in H _s	0s)	Strength of Reaction	Soil Description	
Location	E)	Date:22/12/04 Time:	Date:22/12/04 Time:	Date: Time:	Date: Time:	(1,2,3,4)* F **		
BH 202	SPT 13.0	6.6	6.2			7-	Sand	
BH 202	SPT 14.0	6.8	6.2			~	Sand	
BH 202	SPT 15.0	7.0	6.2			+	Sand	
BH 203	0.5	7.5	6.0			-	Sand	
BH 203	1.0	7.2	6.1			-	Sand	
BH 203	1.5	7.3	6.0			~	Sand	
BH 203	2.0	7.3	6.4			1	Sand	
BH 203	3.0	7.7	6.5			1	Sand	
BH 203	4.0	7.4	6.5			-	Sand	
BH 203	5.0	7.4	6.5			-	Sand	
BH 203	6.0	7.4	6.8			~	Sand	
BH 203	7.0	7.9	74			-	Sand	
BH 203	8.0	7.7	7.5			~	Sand with Shells	1
BH 203	9.0	7.5	6.8			~	Fine Gravely Sand	
Legend: * 1 d	enotes no or s enotes modera	light effervescence ate effervescence				Operator:	LMcK/AC	
4 4 α α α α α	enotes vigorot enotes "volcan after reaction	<i>is effervescence</i> io <i>" ie. very vigorous effer</i> number indicates a but	/escence, gas e obling/frothy re	<i>volution and hee</i> action (organic	rt S	Date:	23 Dec. 04	
			,					

Rev0/March 2003

CM ASS/Table 2

() Douglas Geotechnics - Envir	Bartners				RESULTS C	F Acid Sulfa	TE SCREENING TESTS	1
Client: Project:	Cleary B Propose	sros (Bombo) Pty Ltd d Northern Extension		roject No: Meter:	37673 □ TPS with	Ionode IJ46/WF	280 pH/Temp. Electrode	
Project Location	: Gerroa S Gerroa	Sand Quarry	0	alibration Buffe				
Sample	Denth	pH _F (in distilled water)		pH _{Fox} (oxidised in H _e C	10	Strength of Reaction	Soil Description	
Location	E)	Date:23/12/04	Date:23/12/04	Date:	Date:	(1,2,3,4)*		
RH 203	001	- IIIIe.	1111e.	2		* -	Fine Gravely Sand	
BH 203	11.0	7.6	7.5	-	-		Sand	1
BH 203	12.0	7.9	7.2			-	Sand	
BH 203	13.0	7.9	7.2			~	Sand with Shells	
BH 203	14.0	7.9	7.0			1	Sand	
BH 203	15.0	7.9	6.9			~	Sand	
BH 204	A/S 0.5	7.3	6.2				Silty Sand	
BH 204	SPT 1.0	7.4	6.2			1	Silt	
BH 204	A/S 1.0	7.4	6.5			-	Silty Sand	
BH 204	A/S 1.5	7.4	6.2			~	Silty Sand	
BH 204	SPT 2.0	7.4	6.1			~	Silty Sand	- I
BH 204	SPT 3.0	7.3	6.1			4	Silty Sand	1
BH 204	SPT 4.0	7.2	6.1			~	Silty Sand	
BH 204	SPT 5.0	7.9	7.9			L	Sand	
Legend: * 1 2	denotes no or si denotes modera	light effervescence ate effervescence				Operator:	LMcK/AC	
, 4 Γ	denotes vigurut. denotes "volcan	is enervescence 10" ie. very vigorous effer	vescence, gas e	volution and hea		Date:	23 Dec. 04	
⊷ ἔ	after reaction	number indicates a pul	bbling/trothy re	action (organics	~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~			

RESULTS OF ACID SULFATE SCREENING TESTS

CM ASS/Table 2

Rev0/March 2003

Douglas Geotechnics - Enviro	Partners nment · Groundwater				RESULTS C	F ACID SULFA	TE SCREENING TESTS
Client: Project:	Cleary B Propose	sros (Bombo) Pty Ltd d Northern Extension	L L	roject No: H Meter:	37673 □ TPS with IZI	lonode 1J46/WF	980 pH/Temp. Electrode
Project Location:	Gerroa S Gerroa	Sand Quarry	0	alibration Bu	fer: C pH4 C pH7 pH10		
Sample	Depth	hF (in distilled water)		pH _{Fox} (oxidised in F	(sOs)	Strength of Reaction	Soil Description
Location	(m)	Date:22/12/04 Time:	Date:22/12/04 Time:	Date: Time:	Date: Time:	(1,2,3,4)* F **	
BH 204	SPT 6.0	7.9	7.1			~	Silty Sand
BH 204	SPT 7.0	8.2	7.3			~	Sand
BH 204	SPT 8.0	8.2	7.7			-	Sand
BH 204	10.0	6.3	6.4			-	Sand with Shells
BH 204	11.0	6.2	6.4			~	Sand with Shells
BH 204	12.0	6.5	6.4			~	Sand with Shells
BH 204	13.0	6.6	6.5			~	Sand with Shells
BH 204	14.0	7.7	<u>6.5</u>			0	Clay
BH 204	15.0	78	6.5			ß	Clay
Legend: * 1(2 2 3 3 4 * F	denotes no or s denotes moden denotes vigorot denotes "volcar after reaction	ilight effervescence ate effervescence us effervescence no" ie. very vigorous effe number indicates a bu	rvescence, gas ∈ Ibbling/frothy re	evolution and h	aaf ics	Operator: Date:	LMcK/AC 23 Dec. 04

CM ASS/Table 2

Rev0/March 2003



14 January 2005

TEST REPORT

Douglas Partners Pty Ltd

1 / 1 Luso Drive UNANDERRA NSW 2526

Your Reference:37673, GerroaReport Number:34150

Attention: Arthur Castrissios

Dear Arthur

The following samples were received from you on the date indicated.

Samples: Qty.	3 Soils
Date of Receipt of Samples:	07/01/05
Date of Receipt of Instructions:	07/01/05
Date Preliminary Report Faxed:	Not Issued

These samples were analysed in accordance with your written instructions. A copy of the instructions is attached with the analytical report.

The results and associated quality control are contained in the following pages of this report. Unless otherwise stated, solid samples are expressed on a dry weight basis (moisture has been supplied for your information only), air and liquid samples as received.

Should you have any queries regarding this report please contact the undersigned.

Yours faithfully SGS ENVIRONMENTAL SERVICES



Jacinga Hurst Operations Manager ATA Endored Tex Report his document may not be reproduced except in full. ATA Accredited Laboratory No. 2562 SGS Australia Phy Ltd

ABN 44 000 964 278

Page 1 of 5

Environmental Services Botany Industrial Park Gate 3, Denison Street, Matraville 2036 NSW Australia t +61 (0)2 9666 1426 f +61 (0)2 9666 1364 url www.sgs.com

Inorganics				
Our Reference:	UNITS	34150-1	34150-2	34150-3
Your Reference		Bole 201/1	Bole 201/	Bole 203/
		.5m	3m	4m
Sample Type		soil	soil	soil
pH 1:5 soil:water	pH Units	6.2	6.3	6.7
Chloride, CI* 1:5 soil:water	mg/kg	22	52	1.5
Sulphate, SO4* 1:5 soil:water	mg/kg	6.2	8.1	<2.0



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Method ID	Methodology Summary
SEI-001	pH - Measured using pH meter and electrode in accordance with APHA 20th ED, 4500-H+.
SE1-038	Anions - a range of Anions are determined by Ion Chromatography, in accordance with APHA 20th ED, 4110 -B.



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PROJECT: 37673, Gerroa

REPORT NO: 34150

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QUALITY CONTROL	UNITS	PQL	METHOD	Blank	Duplicate Sm#	Duplicate	Spike Sm#	Matrix Spike % Recovery
Inorganics						Base + Duplicate + %RPD		Duplicate + %RPD
pH 1:5 soil:water	pH Units		SEI-001	[NT]	[TM]	נדא]	Batch	[NT]
Chloride, Cl* 1:5 soil: water	mg/kg	0.5	SEI-038	<0.50	[NT]	[NT]	Batch	106 [N/T]
Sulphate, SO4* 1:5 soil :water	mg/kg	2	SE1-038	<2.0	[NT]	[NT]	Batch	106 j [N/T]



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Result Codes

[INS]	:	Insufficient Sample for this test
[NR]	:	Not Requested
[NT]	:	Not tested

- [HBG] : Results not Reported due to High Background Interference
 - : Not part of NATA Accreditation
- [N/A] : Not Applicable

Result Comments

Date Organics extraction commenced: N/A

NATA Corporate Accreditation No. 2562, Site No 4354

Note: Test results are not corrected for recovery (excluding Dioxins/Furans and PAH in XAD and PUF).

Quality Control Protocol

Reagent Blank: Sample free reagents carried through the preparation/extraction/digestion procedure and analysed at the beginning of every sample batch analysis. For larger projects, a reagent blank is prepared and analysed with every 20 samples.

Duplicate: A separate portion of a sample being analysed which is treated the same as the other samples in the batch. A duplicate is prepared at least every 20 samples.

Matrix Spike Duplicates: Sample replicates spiked with identical concentrations of target analyte(s). The spiking occurs during the sample preparation and prior to the extraction/digestion procedure. They are used to document the precision and bias of a method in a given sample matrix. Where there is not enough sample available to prepare a spiked sample, another known soil/sand or water (or Milli-Q water) may be used. A duplicate spiked sample is prepared at least every 20 samples. Surrogate Spike: Added to all samples requiring analysis for organics (where relevant) prior to extraction. Used to determine the extraction efficiency. They are organic compounds which are similar to the target analyte(s) in chemical composition and behaviour in the analytical process, but which are not normally found in environmental samples. Internal Standard: Added to all samples requiring analysis for organics (where relevant) after the extraction process; the compounds serve to give a standard of retention time and response, which is invariant from run-to-run with the instruments. Control Standards: Prepared from a source independent of the calibration standards. At least one control standard is included in each run to confirm calibration validity.

Additional QC Samples: A calibration standard and blank are run after every 20 samples of an instrumental analysis run to assess analytical drift.



NATA Endorsed Test Report This document may not be reproduced except in full NATA Accredited Laboratory No. 2582 Page 5 of 5

⊺est Type I	sPOCAS		
Order No	52429	Job No: 37673	
Reference	Proposed Nor	rthern Extension	
Sample Name	Pit 201/5m		
Sample No.	86307		
Date Received	05/01/2005	Total No Pages:	1 of 1
Client:	Douglas Parti	ners (Wollongong)
	Chris Wing/A	rthur Castrissois	
	PO Box 486		
	UNANDERR.	A NSW	



Sydney Environmental and Soil Laboratory

Specialists in Soil Chemistry and Agronomy

Sydney Environmental and Soil Laboratory Pty Ltd ABN 70 106 810 708 16 Chilvers Road Thomleigh NSW 2120 Australia Address Mail to PO Box 357 Pennant Hills NSW 1715 Telephone:(02) 9980 6554 Facsimile:(02) 9484 2427 Web: www.sesl.com.au

Tests are performed under a quality system certified as complying with ISO 9002. Results & Conclusions assume that sampling is representative. This document shall not be reproduced except in full

TEST	RESULT	COMMENTS
pH in KCl	9.68	Strongly alkaline
pH in H_2O_2	7.70	alkaline some pH drop
Δ pH unit	1.98	
Acidity Trail		
TPA mol H+/t	<2	
TAA mol H+/t	<2	
TSA mol H+/t	< 2	no excess acidity
Sulphur Trail		
% S _P	.09	
% S _{KCl}	.06	some sulphate present
% S _{POS}	.03	some sulphate released on oxidation
Derived Values		
% S _{TPA} *	< 0.01	no excess sulphuric acidity
Lime Requirement (kg/tonne) **	-0.20	

2526

* TPA equivalent S%, where 1% sulphide produces 623.7 mole H* / tonne soil.

** Includes a safety factor of 1.5.

Recommendations

While there is some sulphate release on oxidation the sample clearly has sufficient excess alkali to neutralise any acid release. The material does not need to be managed as a potential acid sulphate risk.

Explanation of the Methods:

Explanation of the Methods: Ahern CR, Blunden B and Stone Y (eds.) (1998). Acid Sulphate Soils Laboratory Methods Guidelines Published by the Acid Sulphate Soil Managemen Advisory committe, Wollongbar, NSW, Australia

Checked by S. Leake

Test Type I	sPOCAS		
Order No	52429	Job No: 37673	
Reference	Proposed No	rthern Extension	
Sample Name	Pit 201/ 13m		
Sample No.	86308		
Date Received	05/01/2005	Total No Pages:	1 of 1
Client:	Douglas Part	ners (Wollongong	3)
	Chris Wing/A	rthur Castrissois	
	PO Box 486		
	UNANDERR	A NSW	



Sydney Environmental and Soil Laboratory

Specialists in Soil Chemistry and Agronomy

Sydney Environmental and Soil Laboratory Pty Ltd ABN 70 106 810 708 16 Chilvers Road Thornleigh NSW 2120 Australia Address Mail to PO Box 357 Pennant Hills NSW 1715 Telephone: (02) 9980 6554 Facsimile: (02) 9484 2427 Web: www.sesi.com.au Email: sesi@sesi.com.au

Tests are performed under a quality system certified as complying with ISO 9002. Results & Conclusions assume that exampling is representative _____This document shall not be reproduced except in full

TEST	RESULT	COMMENTS
pH in KCl	9.14	strongly alkaline
pH in H_2O_2	8.16	alkaline
∆ pH unit	0.98	some pH drop
Acidity Trail		
TPA mol H+/t	<2	
TAA mol H+/t	<2	
TSA mol H+/t	< 2	no excess acidity
Sulphur Trail		
% S _P	.32	
% S _{KCI}	.04	
% S _{POS}	.28	significant release of sulphate on oxidation
Derived Values		
% S _{TPA} *	< 0.01	no net acidity
Lime Requirement (kg/tonne) **	-0.20	

2526

* TPA equivalent S%, where 1% sulphide produces 623.7 mole H⁺ / tonne soil.

** Includes a safety factor of 1.5.

Recommendations

While there is significant sulphate release on oxidation the sample clearly has sufficient excess alkali to neutralise any acid release. The material does not need to be managed as a potential acid sulphate risk.

Explanation of the Methods:

Explanation of the Methods: Ahem CR, Blunden B and Stone Y (eds.) (1998). Acid Sulphate Soils Laboratory Methods Guidelines Published by the Acid Sulphate Soil Management Advisory committe, Wollongbar, NSW, Australia

Checked by S. Leake

Date of Report 14/01/2005

Consultant...... S.Leake

Test Type I	sPOCAS		
Order No	52429	Job No: 37673	
Reference	Proposed No	rthern Extension	
Sample Name	Pit 202/0.5m		
Sample No.	86309		
Date Received	05/01/2005	Total No Pages:	1 of 1
Client:	Douglas Parti	ners (Wollongong)	
	Chris Wing/A	thur Castrissois	
	PO Box 486		
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Tests are performed under a quality system certified as complying with ISO 9002. Results & Conclusions ssume that sampling is representative. This document nt shall not be reproduced except in full

TEST	RESULT	COMMENTS
pH in KCl	5.62	acidic
pH in H_2O_2	5.94	no pH drop
∆ pH unit	-0.32	
Acidity Trail		
TPA mol H+/t	<2	
TAA mol H+/t	10	some actual acidity, very mild
TSA mol H+/t	< 2	
Sulphur Trail		
% S _P	.17	
% S _{KCI}	.09	some actual acid
% S _{POS}	.08	some sulphuric acidity
Derived Values		
% S _{TPA} *	< 0.01	no net excess of acidity
Lime Requirement (kg/tonne) **	-0.30	

2526

* TPA equivalent S%, where 1% sulphide produces 623.7 mole H⁺ / tonne soil.

** Includes a safety factor of 1.5.

Recommendations

This soil shows some potential sulphidic activity but pH does not decline (in fact increases slightly). We can only assume that the soil has sufficient buffer capacity or lime content to eliminate any pH drop. The soil does not need to be managed as a potential acid sulphate risk.

Explanation of the Methods: Ahem CR, Blunden B and Stone Y (eds.) (1998). Acid Sulphate Soils Laboratory Methods Guidelines Published by the Acid Sulphate Soil Manager Advisory committe, Wollongbar, NSW, Avetralia

Checked by S. Leake



ΞŔ. Consultant.... S.Leaké

⊺est Type I	sPOCAS		
Order No	52429	Job No: 37673	
Reference	Proposed Nor	thern Extension	
Sample Name	Pit 203/5m		
Sample No.	86310		
Date Received	05/01/2005	Total No Pages:	1 of 1
Client:	Douglas Partr	ners (Wollongong)	l
	Chris Wing/Ai	thur Castrissois	
	PO Box 486		
	UNANDERR/	A NSW	



Sydney Environmental and Soil Laboratory Specialists in Soil Chemistry and Agronomy Sydney Environmental and Soil Laboratory Pty Ltd ABN 70 106 810 708 16 Chilvers Road Thornleigh NSW 2120 Australia Address Mail to PO Box 357 Pennant Hills NSW 1715 Telephone: (02) 9980 6554 Facsimile: (02) 9484 2427 Web: www.sesl.com.au Email: sesl@sesl.com.au

Tests are performed under a quality system certified as complying with ISO 9002. Results & Conclusions assume that sampling is representative. This document shall not be reproduced except in full

TEST	RESULT	COMMENTS
pH in KCl	5.79	slightly acidic
pH in H_2O_2	5.35	some pH decline
∆ pH unit	0.44	
Acidity Trail		
TPA mol H+/t	<2	
TAA mol H+/t	2	some actual acidity
TSA mol H+/t	< 2	
Sulphur Trail		
% S _P	.09	
% S _{KCl}	.08	some actual sulphuric acidity
% S _{POS}	.01	very little potential sulphuric acidity
Derived Values		
% S _{TPA} *	< 0.01	no net acidity
Lime Requirement (kg/tonne) **	0.00	

2526

* TPA equivalent S%, where 1% sulphide produces 623.7 mole H+ / tonne soil.

** Includes a safety factor of 1.5.

Recommendations

This soil shows only a very slight potential acidity but pH does not decline to risky levels. Essentially the material is not in the potential acid sulphate category.

Explanation of the Methods:

Explanation of the Methods: Ahern CR, Blunden B and Stone Y (eds.) (1999). Acid Sulphate Soils Laboratory Methods Guidelines Published by the Acid Sulphate Soil Management Advisory committe, Wollongbar, NSW, Australia

Checked by S. Leake

Date of Report 14/01/2004

Test Type I	sPOCAS		
Order No	52429	Job No: 37673	
Reference	Proposed Nor	thern Extension	
Sample Name	Pit 204/14m		
Sample No.	86311		
Date Received	05/01/2005 1	Fotal No Pages:	1 of 1
Client:	Douglas Partr	ners (Wollongong)	
	Chris Wing/Ar	thur Castrissois	
	PO Box 486		
	UNANDERRA	A NSW	



Sydney Environmental and Soil Laboratory Specialists in Soil Chemistry and Agronomy Sydney Environmental and Soil Laboratory Pty Ltd ABN 70 106 810 708 16 Chilvers Road Thornleigh NSW 2120 Australia Address Mail to PO Box 357 Pennant Hills NSW 1715 Telephone: (02) 9980 6554 Facsimile: (02) 9484 2427 Web: www.sesi.com.au Email: sesi@sesi.com.au

Tests are performed under a quality system certified as complying with ISO 9002.

TEST	RESULT	COMMENTS
pH in KCl	6.01	slightly acidic
pH in H_2O_2	2.39	very significant pH decline
∆ pH unit	3.62	
Acidity Trail		
TPA mol H+/t	296	
TAA mol H+/t	2	little actual acidity
TSA mol H+/t	294	very significant potential sulphuric acidity
Sulphur Trail		
% S _p	1	
% S _{KCl}	.11	some actual sulphate
% S _{POS}	.89	very significant potential sulphuric acidity
Derived Values		
% S _{TPA} *	.47	some buffer capacity present in the sample
Lime Requirement (kg/tonne) **	22.20	

2526

hall not be reproduced

* TPA equivalent S%, where 1% sulphide produces 623.7 mole H* / tonne soil.

** Includes a safety factor of 1.5.

Recommendations

This sample is a significant potential acid sulphate risk. In the absence of any neutralising or buffer capacity the derived value of %STPA should equal %Sp. The caft that it does not indicates the presence of buffer capacity or lime in the sample. Even so there is still a significant excess of acidity and the sample must be treated as a severe acid sulphate risk. The lime requirement will theoretically gfive a resting pH of 6.5 in the oxidised sample.

Explanation of the Mathods:

Explanation of the Methods: Ahern CR, Blunden B and Stone Y (eds.) (1998). Acid Sulphate Soils Laboratory Methods Guidelines Published by the Acid Sulphate Soil Managgment Advisory committe, Wolfongbar, NSW, Australia

te of Report 14/01/2004

Consultant. S.Lea

APPENDIX E Results of Tests of Surface and Groundwater

Analytical Report - Enviro-Managers									
Client:	Cleary Bros (Bombo) Pty Ltd								
	Springhill Rd								
Contact Name:	Mr Ron Bryant								
Client Reference:	Gerroa Bores								

NR = No Result - Dry

Notes	Report Number:	W05/0186	W05/0186	W05/0186	W05/0186	W05/0186	W05/0186	W05/0186	W05/0671	W05/0671	W05/0671	W05/0671	W05/0671	W05/0671	W05/0671	W05/0671	W05/0671	W05/0671
Results:	Sample Received:	20/01/05	20/01/05	20/01/05	20/01/05	20/01/05	20/01/05	20/01/05	24/02/05	24/02/05	24/02/05	24/02/05	24/02/05	24/02/05	24/02/05	24/02/05	24/02/05	24/02/05
Client Id		Ex Works	BH 1	BH 7	BH 9	BH 11	B/Angel Creek	BH 12	Ex.Works	Bore Hole 1	Bore Hole 2	Bore Hole 4	Bore Hole 5	Bore Hole 6	Bore Hole 7	Bore Hole 9	Bore Hole 11	lue Angle Cre
Laboratory Id		W11016/001	W11016/002	W11016/003	W11016/004	W11016/005	W11016/006	W11016/007	W11511/001	W11511/002	W11511/003	W11511/004	W11511/005	W11511/006	W11511/007	W11511/008	W11511/009	W11511/010
Conductivity (uS/cm)																		
Method:APHA 2510 B	Units:uS/cm@25℃	560	1030	160	860	1110	+20000	430	540	370	NR	NR	NR	NR	150	380	1510	810
Groundwater level (RL)																		
Method:	Units:m	-	0.48	-0.46	-1.15	-1.53	-	-0.40	-	1.83	-	-	-	-	-0.01	-0.60	-1.28	-
pН																		
Method:APHA 4500 H B	Units:pH units	7.8	6.6	5.7	6.4	6.4	6.9	5.4	7.5	5.7	-	-	-	-	5.6	6.5	4.9	6.3

Notes	Report Number:	W05/2033-1	W05/2033-1	W05/2033-1	W05/2033-1	W05/2033-1	W05/2033-1	W05/2033-1	W05/2033-1	W05/2033-1	W05/2033-1	W05/2033-1	W05/2033-1	W05/2358	W05/2358	W05/2358	W05/2358	W05/2358
Results:	Sample Received:	26/05/05	26/05/05	26/05/05	26/05/05	26/05/05	26/05/05	26/05/05	26/05/05	26/05/05	26/05/05	26/05/05	26/05/05	22/06/05	22/06/05	22/06/05	22/06/05	22/06/05
Client Id		Ex-Works	BH 1	BH 2	BH 4	BH 5	BH 6	BH 7	BH 9	BH 11	3/Angel Cree	BH 12	BH 14	Ex Works	BH 1	BH 2	BH 4	BH 5
Laboratory Id		W12828/001	W12828/002	W12828/003	W12828/004	W12828/005	W12828/006	W12828/007	W12828/008	W12828/009	W12828/010	W12828/011	W12828/012	W13143/001	W13143/002	W13143/003	W13143/004	W13143/005
Conductivity (uS/cm)																		
Method:APHA 2510 B	Units:uS/cm@25℃	540	340	NR	NR	NR	NR	190	250	1900	4240	380	NR	560	360	NR	NR	NR
Groundwater level (RL)																		
Method:	Units:m	-	+1.73	-	-	-	-	-0.16	-0.75	-1.33	-	-0.05	-	-	1.33	-	-	-
pН																		
Method: APHA 4500 H B	Units nH units	6.8	5.8	-		-	-	5.8	61	46	64	5.8	-	67	5.8	-	-	-

Notes	Report Number:	W05/3572-2	W05/3572-2	W05/3572-2	W05/3572-2	W05/3572-2	W05/3572-2	W05/3572-2	W05/3572-2	W05/3572-2	W05/3572-2	W05/3572-2	W05/3572-2	W05/3572-2	W05/3963	W05/3963	W05/3963	W05/3963
Results:	Sample Received:	21/09/05	21/09/05	21/09/05	21/09/05	21/09/05	21/09/05	21/09/05	21/09/05	21/09/05	21/09/05	21/09/05	21/09/05	21/09/05	21/10/05	21/10/05	21/10/05	21/10/05
Client Id		Ex.Works	BH 1	BH 1A	BH 4	BH 5	BH 6	BH 7	BH 9	BH 11	3/Angel Cree	BH 12	BH 2A	BH 3A	Ex.Works	BH 1	BH 1A	BH 4
Laboratory Id		W14389/001	W14389/002	W14389/003	W14389/004	W14389/005	W14389/006	W14389/007	W14389/008	W14389/009	W14389/010	W14389/011	W14389/012	W14389/013	W14768/001	W14768/002	W14768/003	W14768/004
Conductivity (uS/cm)																	1	
Method:APHA 2510 B	Units:uS/cm@25℃	580	640	NR	750	NR	NR	160	270	1280	18030	770	NR	NR	600	1010	Dry	Dry
Groundwater level (RL)																	1	
Method:	Units:m	-	1.39	-	-0.33	-	-	-0.29	-0.87	-1.42	-	-0.26	-	-	-	1.05	-	-
pН																	1	
Method:APHA 4500 H B	Units:pH units	6.7	5.8	-	6.9	-	-	5.5	6.0	5.3	6.8	5.3	-	-	6.9	6.0	-	-

YEARLY SAMPLING

	Report Number:	W05/4442	W05/4442	W05/4442	W05/4442	W05/4442	W05/4442	W05/4442	W05/4442	W05/4442	W05/4442	W05/4442	W05/4442	W05/4442
Results:	Sample Received:	21/11/05	21/11/05	21/11/05	21/11/05	21/11/05	21/11/05	21/11/05	21/11/05	21/11/05	21/11/05	21/11/05	21/11/05	21/11/05
Client Id		Ex-Works	BH 1	BH 3A	BH 4	BH 5	BH 6	BH7	BH 9	BH 11	ue Angle Cre	BH 12	BH 2A	BH 1A
Laboratory Id		W15175/001	W15175/002	W15175/003	W15175/004	W15175/005	W15175/006	W15175/007	W15175/008	W15175/009	W15175/010	W15175/011	W15175/012	W15175/013
CI : SO4 Ratio														
Method:	Units:-	0.53	4.4	NR	5.1	NR	NR	3.3	0.88	3.6	7.4	0.56	NR	NR
Conductivity (uS/cm)														
Method:APHA 2510 B	Units:uS/cm@25℃	690	590	-	1080	-	-	170	410	1180	>20,000	570	-	-
Groundwater level (RL)														
Method:	Units:m	-	1.33	-	0.57	-	-	-0.42	-1.08	-1.30	-	-0.31	-	-
рН														
Method:APHA 4500 H B	Units:pH units	7.7	6.9	-	7.9	-	-	6.9	7.3	6.8	7.7	6.5	-	-

Analytical Report - Enviro-	Managers
Client:	Cleary Bros (Bombo) Pty Ltd
	Springhill Rd
Contact Name:	Mr Ron Bryant
Client Reference:	Gerroa Bores

NR = No Result - Dry

Notes	Report Number:	W05/0671	W05/0671	W05/1086-1	W05/1086-1	W05/1086-1	W05/1086-1	W05/1086-1	W05/1086-1	W05/1086-1	W05/1086-1	W05/1086-1	W05/1086-1	W05/1086-1	W05/1086-1	W05/1620	W05/1620	W05/1620
Results:	Sample Received:	24/02/05	24/02/05	22/03/05	22/03/05	22/03/05	22/03/05	22/03/05	22/03/05	22/03/05	22/03/05	22/03/05	22/03/05	22/03/05	22/03/05	27/04/05	27/04/05	27/04/05
Client Id		Bore Hole 12	Bore Hole 14	Ex Works	BH 1	BH 2	BH 4	BH 5	BH 6	BH 7	BH 9	BH 11	lue Angle cree	BH 12	BH 14	Ex-Works	BH1	BH2
Laboratory Id		W11511/011	W11511/012	W11945/001	W11945/002	W11945/003	W11945/004	W11945/005	W11945/006	W11945/007	W11945/008	W11945/009	W11945/010	W11945/011	W11945/012	W12413/001	W12413/002	W12413/003
Conductivity (uS/cm)																		
Method:APHA 2510 B	Units:uS/cm@25℃	350	NR	530	290	NR	NR	NR	NR	160	450	820	9140	330	NR	510	300	NR
Groundwater level (RL)																		
Method:	Units:m	-0.15	-	-	1.73	-	-	-	-	-0.16	-0.80	-1.48	-	-0.20	-	-	+1.53	-
pН																		
Method:APHA 4500 H B	Units:pH units	5.3	-	6.6	6.0	-	-	-	-	5.9	6.9	5.9	6.7	5.3	-	7.3	6.5	-

Notes	Report Number:	W05/2358	W05/2358	W05/2358	W05/2358	W05/2358	W05/2358	W05/2358	W05/2774	W05/2774	W05/2774	W05/2774	W05/2774	W05/2774	W05/2774	W05/2774	W05/2774	W05/2774
Results:	Sample Received:	22/06/05	22/06/05	22/06/05	22/06/05	22/06/05	22/06/05	22/06/05	21/07/05	21/07/05	21/07/05	21/07/05	21/07/05	21/07/05	21/07/05	21/07/05	21/07/05	21/07/05
Client Id		BH 6	BH 7	BH 9	BH 11	ue Angel Cre	BH 12	BH 14	Ex Works	BH 1	BH 2	BH 4	BH 5	BH 6	BH 7	BH 9	BH 11	ue Angel Cre
Laboratory Id		W13143/006	W13143/007	W13143/008	W13143/009	W13143/010	W13143/011	W13143/012	W13573/001	W13573/002	W13573/003	W13573/004	W13573/005	W13573/006	W13573/007	W13573/008	W13573/009	W13573/010
Conductivity (uS/cm)																		
Method:APHA 2510 B	Units:uS/cm@25℃	NR	210	220	900	>20,000	NR	NR	550	440	NR	740	100	NR	160	220	2150	5200
Groundwater level (RL)																		
Method:	Units:m	-	-0.31	-0.95	-0.88	-	-	-	-	1.73	-	0.07	0.23	-	0.09	-0.50	-1.48	-
pH																		
Method:APHA 4500 H B	Units:pH units	-	5.6	6.0	5.4	6.5	-	-	7.0	5.7	-	6.4	5.0	-	5.4	5.7	4.9	6.4

Notes	Report Number:	W05/3963	W05/3963	W05/3963	W05/3963	W05/3963	W05/3963	W05/3963	W05/3963	W05/3963	W05/4819	W05/4819	W05/4819	W05/4819	W05/4819	W05/4819	W05/4819	W05/4819
Results:	Sample Received:	21/10/05	21/10/05	21/10/05	21/10/05	21/10/05	21/10/05	21/10/05	21/10/05	21/10/05	20/12/05	20/12/05	20/12/05	20/12/05	20/12/05	20/12/05	20/12/05	20/12/05
Client Id		BH 5	BH 6	BH 7	BH 9	BH 11	ue Angle Cre	BH 12	BH 2A	BH 3A	Ex-Works	BH 1	BH 1A	BH 4	BH 5	BH 6	BH 7	BH 9
Laboratory Id		W14768/005	W14768/006	W14768/007	W14768/008	W14768/009	W14768/010	W14768/011	W14768/012	W14768/013	W15693/001	W15693/002	W15693/003	W15693/004	W15693/005	W15693/006	W15693/007	W15693/008
Conductivity (uS/cm)																		
Method:APHA 2510 B	Units:uS/cm@25℃	Dry	Lost	170	260	1010	>20,000	620	Dry	Dry	670	470	Dry	1110	Dry	NR	140	380
Groundwater level (RL)																		
Method:	Units:m	-	-	-0.41	-1.07	-1.33	-	-0.37	-	-	-	1.21	-	-0.43	-	-	-0.40	-0.98
pH																		
Method:APHA 4500 H B	Units:pH units	-	-	5.6	5.6	5.2	6.6	5.3	-	-	7.7	6.1	-	7.2	-	-	5.9	6.7

YEARLY SAMPLING

	Report Number:
Results:	Sample Received:
Client Id	
Laboratory Id	
CI : SO4 Ratio	
Method:	Units:-
Conductivity (uS/cm)	
Method:APHA 2510 B	Units:uS/cm@25℃
Groundwater level (RL)	
Method:	Units:m
рН	
Method:APHA 4500 H B	Units:pH units

Analytical Report - Enviro-	Managers
Client:	Cleary Bros (Bombo) Pty Ltd
	Springhill Rd
Contact Name:	Mr Ron Bryant
Client Reference:	Gerroa Bores

NR = No Result - Dry

Notes	Report Number:	W05/1620	W05/1620	W05/1620	W05/1620	W05/1620	W05/1620	W05/1620	W05/1620	W05/1620
Results:	Sample Received:	27/04/05	27/04/05	27/04/05	27/04/05	27/04/05	27/04/05	27/04/05	27/04/05	27/04/05
Client Id		BH4	BH5	BH6	BH7	BH9	BH11	B/Angle Creek	BH12	BH14
Laboratory Id		W12413/004	W12413/005	W12413/006	W12413/007	W12413/008	W12413/009	W12413/010	W12413/011	W12413/012
Conductivity (uS/cm)										
Method:APHA 2510 B	Units:uS/cm@25℃	NR	NR	NR	140	400	790	7380	410	NR
Groundwater level (RL)										
Method:	Units:m	-	-	-	-0.31	-0.95	-1.53	-	-1.30	-
pН										
Method:APHA 4500 H B	Units:pH units	-	-	-	5.6	6.5	5.6	6.9	5.9	-

Notes	Report Number:	W05/2774	W05/2774	W05/3172	W05/3172	W05/3172	W05/3172	W05/3172	W05/3172	W05/3172	W05/3172	W05/3172	W05/3172	W05/3172	W05/3172
Results:	Sample Received:	21/07/05	21/07/05	22/08/05	22/08/05	22/08/05	22/08/05	22/08/05	22/08/05	22/08/05	22/08/05	22/08/05	22/08/05	22/08/05	22/08/05
Client Id		BH 12	BH 14	WM1A	xisting Work	WM1	WM2A	WM4	WM5	WM3A	WM7	WM9	WM11	ue Angle Cre	WM12
Laboratory Id		W13573/011	W13573/012	W13973/001	W13973/002	W13973/003	W13973/004	W13973/005	W13973/006	W13973/007	W13973/008	W13973/009	W13973/010	W13973/011	W13973/012
Conductivity (uS/cm)															
Method:APHA 2510 B	Units:uS/cm@25℃	NR	NR	NR	560	670	NR	780	NR	NR	160	250	1360	8540	760
Groundwater level (RL)															
Method:	Units:m	-	-	-	-	1.12	-	0.78	-	-	-0.14	-0.72	-1.29	-	-0.17
pН															
Method:APHA 4500 H B	Units:pH units	-	-	-	7.0	6.0	-	6.8	-	-	5.6	5.5	4.5	6.7	5.4

Notes	Report Number:	W05/4819	W05/4819	W05/4819	W05/4819	W05/4819
Results:	Sample Received:	20/12/05	20/12/05	20/12/05	20/12/05	20/12/05
Client Id		BH 11	ue Angle Cre	BH 12	BH 2A	BH 3A
Laboratory Id		W15693/009	W15693/010	W15693/011	W15693/012	W15693/013
Conductivity (uS/cm)						
Method:APHA 2510 B	Units:uS/cm@25℃	1230	17420	590	Dry	Dry
Groundwater level (RL)						
Method:	Units:m	-1.44	-	-0.17	-	-
pН						
Method:APHA 4500 H B	Units:pH units	5.7	7.0	5.3	-	-

YEARLY SAMPLING

	Report Number:
Results:	Sample Received:
Client Id	
Laboratory Id	
CI : SO4 Ratio	
Method:	Units:-
Conductivity (uS/cm)	
Method:APHA 2510 B	Units:uS/cm@25 °C
Groundwater level (RL)	
Method:	Units:m
рН	
Method:APHA 4500 H B	Units:pH units

TABLE 1: Surface Water Analytical Results Foys Swamp, Blue Angle Creek and Gerroa Sand Quarry

Toys Swallip, Dide Angle Cleek and			у	1			1			r	r	1	r	1	T	r	r	1	1	1	r
				Sample ID	M.DRAIN-1	M.DRAIN-1	M.DRAIN-1	M.DRAIN-1	M.DRAIN-1	M.DRAIN-1	M.DRAIN-1	M.DRAIN-2	M.DRAIN-2	M.DRAIN-2	M.DRAIN-2	M.DRAIN-2	M.DRAIN-2	M.DRAIN-2	BA Creek	BA Creek	BA Creek
					Main Drain-	Main Drain-	Main Drain-	Main Drain-	Main Drain-	Main Drain-	Main Drain-	Main Drain-	Main Drain-	Main Drain-	Main Drain- dn	Main Drain- dn	Main Drain- dn	Main Drain- dn	Blue Angle	Blue Anale	Blue Angle
Cleary Bros (Bombo) Pty Ltd	Water Mor	nitoring Pro	gram	Sample ID	up stream	up stream	up stream	up stream	up stream	up stream	up stream	dn stream	dn stream	dn stream	stream	stream	stream	stream	Creek	Creek	Creek
		r	1		•	•	•	•	•		•								-		
	Units	LOR	ANZECC 20	00 Guidelines	Wet Weather	Dry Weather	Dry Weather	Dry Weather	Dry Weather	Dry Weather	Dry Weather	Wet Weather	Dry Weather	Dry Weather	Dry Weather	Dry Weather	Dry Weather	Dry Weather	Wet Weather	Dry Weather	Dry Weather
Matala (tatal)			Marine	Fresh	07/04/05	00/04/05	0/00/05	0/00/05	04/00/05	04/40/00	04/44/05	07/04/05	00/04/05	0/00/05	0/00/05	04/00/05	04/40/00	04/44/05	0/00/05	00/04/05	0/00/05
	m a/l	0.0001	ID	0.055	27/01/05	28/04/05	2/06/05	3/08/05	21/09/05	21/10/06	21/11/05	27/01/05	28/04/05	2/06/05	3/08/05	21/09/05	21/10/06	21/11/05	3/02/05	28/04/05	2/06/05
	IIIg/L	0.0001	ID	0.055	0.004		0.99	2.1			0.20	0.000		0.001	0.004			0.12	0.000		0.39
Arsenic	mg/L	0.001	ID	0.013	<0.001		<0.001	<0.001	-	-	<0.001	0.002		<0.001	<0.001	-	-	<0.001	0.002		0.004
Beryllium	mg/L	0.001	ID	ID	<0.001							<0.001							0.001		
Barium	mg/L	0.001	ID	ID	0.016							0.015							0.014		
Cadmium	mg/L	0.0001	0.0007	0.0002	0.0002		<0.0001	<0.0001			<0.001	0.0003		<0.0001	<0.0001			<0.001	0.0004		<0.0001
Chromium (Total)	mg/L	0.001	0.0274	ID	0.003		<0.001	0.001				0.003		<0.001	<0.001				0.003		<0.001
Cobalt	mg/L	0.001	0.001	ID	0.01							0.009							0.004		
Copper	mg/L	0.001	0.0013	0.0014	<0.001		0.003	0.006			<0.001	<0.001		0.003	0.002			<0.001	0.01		0.006
Lead	mg/L	0.001	0.0044	0.0034	<0.001		<0.001	<0.001			< 0.001	<0.001		<0.001	<0.001			<0.001	0.01		<0.001
Zinc	mg/L	0.005	0.015	0.008	0.013		0.012	0.031			< 0.005	0.012		0.023	0.009			< 0.005	0.023		0.022
Manganese	mg/L	0.001	ID	1.9	0.596							0.543							0.14		
Nickel	mg/L	0.001	0.007	0.011	0.01							0.009							0.006		
Vanadium	ma/L	0.01	0.1	ID	< 0.01							< 0.01							< 0.01		
Total Iron	ma/L	0.005	ID	ID	0.75		0.71	2.48			0.30	0.42		0.49	0.52			0.11	<0.1		2.07
Mercury	mg/l	0.0001	0.0001	0,00006	<0.0001		<0.0001	<0.0001			<0.0005	<0.0001		<0.0001	<0.0001			<0.0005	0.0001		<0.0001
		0.000.	0.0001	0.00000	1010001			1010001							1010001			1010000	0.0001		
Weak Acid Dissociable Cvanide	ma/l	0.005	0.004	0.007								<0.0050									
Treak Acia Dissectable Oyunae	iiig/E	0.000	0.004	0.007								<0.0000									
Nutrients				1								1							1		
Fluoride	ma/l	0.1		1															0.1		
Ammonia as N	mg/L	0.01	0.91	0.9	0.052	0.028	0.073	0.084	0.18	<0.02	0.25	0.096	0.031	0.047	0.02	0.17	0.02	0.19	0.048	0 282	0.53
Nitrato as N	mg/L	0.01	0.91	0.3	<0.002	<0.020	0.076	0.004	<0.10	0.02	<0.20	<0.000	<0.001		0.02	<0.04	0.02	<0.01	<0.040	0.202	~0.010
Nitrito as N	mg/L	0.01		0.7	<0.010	<0.010	~0.020	0.014	<0.04	0.03	0.04	<0.010	<0.010	<0.010	<0.023	<0.04	0.03	0.005	0.014	<0.002	0.026
Total Kieldehl Nitregen es N	mg/L	0.01			0.010	0.60	2.40	0.013	<0.002	0.014	0.004	0.010	<0.010	0.010	<0.010	<0.002	0.014	0.003	1.90	1.00	0.020
Total Recentering on D	mg/L	0.10		0.05	0.80	0.00	2.40	2.20	0.04	0.53	0.03	0.70	0.30	0.90	0.00	0.80	0.40	0.47	1.60	0.00	2.30
Reactive Bhasekerry	mg/L	0.01		0.05	0.04	0.01	0.25	1.02	<0.005	<0.005	< 0.005	0.01	<0.010	0.02	<0.01	<0.005	<0.005	<0.005	0.10	0.06	0.20
Reactive Phosphorus	mg/L	0.01			<0.010	<0.01	<0.010	<0.010	<0.004	<0.004	<0.004	0.912	<0.010	<0.010	<0.010	<0.004	<0.004	<0.004		<0.010	<0.010
		0.04				0.47	0.70	0.00	7.00	7.00	0.00		0.00	0.74	0.00	7.00	7.00	0.00		0.00	0.07
PH (lab)	pH Unit	0.01				6.17	6.72	6.33	7.20	7.00	6.80		6.90	6.71	6.68	7.00	7.00	6.90		6.80	6.87
Total Dissolved Solids (TDS)	mg/L	1			552	2460	646	1230	4515.8	5808.9	3577.8		1240	644	1730	3912.8	5453.8	4127.2	303	2330	18500
Electrical Conductivity	uS/cm	1				_			6740	8670	5340			-		5840	8140	6160			
Suspended Solids (SS)	mg/L	1			11	7		3				6	10	-	9				23	13	
Total Hardness	mg/L	1			164														73		
Major lons							07				50								40		005
	mg/L	1		l	28		37			-	53						-	63	13		225
Magnesium	mg/L	1			20		26				93							110	10		587
Sodium	mg/L	1			87		134				669			-				789	39		4460
Potassium	mg/L	1			10		10				31			-				36	6		176
Bicarbonate as CaCO3	mg/L	1		ļ	2		31												10		90
Total Alkalinity	mg/L	1			2		31												10		90
Sulphate as SO4	mg/L	1			159		182				293							392	63		1200
Chloride	mg/L	1			130		212				1308							1684	64.5		8930
SAR																					
Calcium + Magnesium (meq/L)					3.04		3.99				10.29							12.19	1.47		59.51
Sodium (meq/L)					3.78		5.83				29.10							34.32	1.70		194.01
SAR= Na / Sqrt (Ca+ Mg) / 2)					1.23		1.41				2.27							2.47	0.86		5.46
SAR- Sodium Absorption Ratio					3.07		4.13				12.83							13.90	1.98		35.57
SAR Hazard Ranking					Low		Low				Med							Med	Low		V High

Note:

SAR Hazard ranking based on Fetter, 1994. Low = 2 to 10, Med = 7 to 18, High= 11 to 26, V High= 26+ nr - no recommended NSW guidelines NA - Not Available TDS= EC*0.67 (approximate) calculation in italics (Data from Sept to Dec05)

3 Exceeds ANZECC 2000 trigger values (marine and/or fresh water)

3

TABLE 1: Surface Water Analytical ResultsFoys Swamp, Blue Angle Creek and Gerroa Sand Quarry

Toys on amp, Blac Angle of certain	001104 00		y								
				Sample ID	BA Creek	BA Creek	BA Creek	BA Creek	SW Drain	SW Drain	SW Drain
Cleary Bros (Bombo) Pty Ltd	Water Moi	nitoring Pro	ogram	Sample ID	Blue Angle Creek	Blue Angle Creek	Blue Angle Creek	Blue Angle Creek	SW Drain	SW Drain	SW Drain
	Units	LOR	ANZECC 20	00 Guidelines	Dry Weather	Dry Weather	Dry Weather	Dry Weather	Wet Weather	Dry Weather	Dry Weathe
Motols (total)			Marine	Fresh	2/02/05	21/00/05	21/10/06	24/44/05	27/04/05	2/06/05	24/44/2005
Metals (total)	ma/l	0.0001	ID	0.055	3/08/05	21/09/05	21/10/06	21/11/05	27/01/05	2/06/05	21/11/2005
Areonic	mg/L	0.0001		0.055	0.003			<0.02	0.004		
Beryllium	mg/L	0.001		0.013	0.003			<0.001	<0.004		
Barium	mg/L	0.001									
Cadmium	mg/L	0.001	0.0007	0.0002	<0.0001			<0.001	<0.013		
Chromium (Total)	mg/L	0.0001	0.0007	0.0002	0.001			<0.001	0.003		
Cobalt	mg/L	0.001	0.0214		0.001				0.003		
Copper	mg/L	0.001	0.0013	0.001/	0.008			<0.001	0.002		
Lead	mg/L	0.001	0.0013	0.0014	<0.000			<0.001	<0.002		
Zinc	mg/L	0.001	0.0044	0.0034	0.016			<0.001	0.412		
Manganese	mg/L	0.000	0.015	1.0	0.010			<0.000	0.003		
Nickel	ma/l	0.001	0.007	0.011			+	<u> </u>	<0.00		
Vanadium	ma/l	0.01	0.007				+	<u> </u>	<0.005		
Total Iron	mg/L	0.005	ID		7 12			0.28	2 16		
Mercury	mg/L	0.0001	0.0001	0,000.6	<0.0001			<0.0005	<0.0001		
	iiig/L	0.0001	0.0001	0.00000	20.0001			<0.0000	CO.0001		
Weak Acid Dissociable Cyanide	mg/L	0.005	0.004	0.007							
Nutrients											
Fluoride	mg/L	0.1									
Ammonia as N	mg/L	0.01	0.91	0.9	0.652	0.22	0.12	0.26	0.074	0.096	0.15
Nitrate as N	mg/L	0.01	ID	0.7	0.231	0.05	0.08	< 0.04	<0.010	0.016	< 0.04
Nitrite as N	mg/L	0.01			0.114	0.005	0.021	0.004	0.012	<0.010	0.11
Total Kjeldahl Nitrogen as N	mg/L	0.10			0.90	0.73	0.58	0.44		1.50	1.5
Total Phosphorus as P	mg/L	0.01		0.05	0.05	< 0.005	<0.005	< 0.005	0.31	0.13	0.05
Reactive Phosphorus	mg/L	0.01			<0.010	<0.004	<0.004	<0.004		0.123	0.043
PH (lab)	nH I Init	0.01			6.89	7.00	6.90	6 70		6.87	75
Total Dissolved Solids (TDS)	ma/l	1			3620	3752	7624.6	4107.1	203	243	520
Electrical Conductivity	uS/cm	1			0020	5600	11380	6130	200	245	520
Suspended Solids (SS)	ma/l	1			26	0000	11000	0100	20		
Total Hardness	mg/L	1			20				52		
Major lons											
Calcium	mg/L	1						63	10		
Magnesium	mg/L	1						111	7		
Sodium	mg/L	1						808	26		
Potassium	mg/L	1						37	7		
Bicarbonate as CaCO3	mg/L	1							39		
Total Alkalinity	mg/L	1							39		
Sulphate as SO4	mg/L	1						345	8		
Chloride	mg/L	1						1635	49		
SAR											
Calcium + Magnesium (meq/L)								12.27	1.07		
Sodium (meq/L)								35.15	1.13		
SAR= Na / Sqrt (Ca+ Mg) / 2)			<u> </u>				ļ	2.48	0.73		
SAR- Sodium Absorption Ratio			ļ	ļ	ļ		ļ	14.19	1.54		
SAR Hazard Ranking								Med	Low		
Note:											

SAR Hazard ranking based on Fetter, 1994. Low = 2 to 10, Med = 7 to 18, High= 11 to 26, V High= 26+ nr - no recommended NSW guidelines NA - Not Available TDS= EC*0.67 (approximate) calculation in italics (Data from Sept to Dec05)



TABLE 1: Surface Water Analytical Results Foys Swamp, Blue Angle Creek and Gerroa Sand Quarry

Foys Swamp, Dive Angle Creek and	Gentua Sa	anu Quan	у							1	1						1	·	
				Sample ID	W Drain	W Drain	W Drain	NW Drain	NW Drain	NW Drain	NW Drain	NW Drain	NW Drain	Dredge Pond	Dredge Pond	Dredge Pond	Dredge Pond	Dredge Pond	Dredge Pond
Cleary Bros (Bombo) Pty Ltd	Water Mo	nitoring Pro	ogram	Sample ID	W Drain	W Drain	W Drain	NW Drain	NW Drain	NW Drain	NW Drain	NW Drain	NW Drain	MD Pond	MD Pond	MD Pond	MD Pond	MD Pond	MD Pond
	Units	LOR	ANZECC 200	00 Guidelines	Wet Weather	Dry Weather	Dry Weather	Wet Weather	Dry Weather	Dry Weather	Dry Weather	Dry Weather	Dry Weather	Wet Weather	Dry Weather	Dry Weather	Dry Weather	Dry Weather	Dry Weather
			Marina	Frosh														()	
Metals (total)			Warne	Tiesii	27/01/2005	2/06/2005	21/11/2005	27/01/2005	2/06/05	3/08/05	21/09/05	21/10/05	21/11/05	27/01/05	2/06/05	3/08/05	21/09/05	21/10/05	21/11/05
Aluminium (PH>6.5,)	mg/L	0.0001	ID	0.055					3.04	2.83			18		1.42	1.28		ļļ	1.9
Arsenic	mg/L	0.001	ID	0.013	0.003			<0.001	<0.001	<0.001			<0.001	0.004	0.003	0.002		ļļ	<0.001
Beryllium	mg/L	0.001	ID	ID	<0.001			0.002						<0.001				ļ]	
Barium	mg/L	0.001	ID	ID	0.03			0.028						0.01				ļ]	
Cadmium	mg/L	0.0001	0.0007	0.0002	0.0005			0.0003	<0.0001	<0.0001			<0.001	0.0002	<0.0001	<0.0001		ļ]	<0.001
Chromium (Total)	mg/L	0.001	0.0274	ID	0.003			0.002	<0.001	<0.001				0.004	0.002	0.001		ļ]	
Cobalt	mg/L	0.001	0.001	ID	<0.001			0.038						<0.001				<u>ا</u> ـــــــــــا	ļ
Copper	mg/L	0.001	0.0013	0.0014	0.025			0.003	0.002	0.004			0.002	<0.001	0.002	0.003		J	<0.001
Lead	mg/L	0.001	0.0044	0.0034	0.001			<0.001	<0.001	<0.001			<0.001	<0.001	<0.001	<0.001		<u>ا</u> ـــــــــــا	<0.001
Zinc	mg/L	0.005	0.015	0.008	0.072			1.72	0.035	0.019			0.053	0.015	0.05	0.012		J	< 0.005
Manganese	mg/L	0.001	ID	1.9	0.004			0.028						0.003					<u> </u>
Nickel	mg/L	0.001	0.007	0.011	<0.01			<0.01						<0.01					<u> </u>
Vanadium	mg/L	0.01	0.1	ID	0.022			0.045						<0.005					<u> </u>
Total Iron	mg/L	0.005	ID	ID	0.83			8.64	0.39	1.4			0.58	0.77	1.14	0.78			0.57
Mercury	mg/L	0.0001	0.0001	0.00006	<0.0001			0.0016	<0.0001	<0.0001			<0.0005	<0.0001	<0.0001	<0.0001		í J	<0.0005
																		l l	
Weak Acid Dissociable Cyanide	mg/L	0.005	0.004	0.007										<0.0050				[]	
																		[]	
Nutrients																			
Fluoride	mg/L	0.1												0.2					
Ammonia as N	mg/L	0.01	0.91	0.9	0.055	0.043	0.35	0.066	0.046	0.059	0.23	<0.02	0.31	0.063	0.038	<0.010	0.20	0.03	0.19
Nitrate as N	mg/L	0.01	ID	0.7	0.011	<0.010	< 0.04	<0.010	0.301	<0.010	<0.04	<0.04	<0.04	0.023	<0.010	0.025	< 0.04	0.04	< 0.04
Nitrite as N	mg/L	0.01			<0.010	<0.010	0.022	<0.010	<0.010	<0.010	<0.002	0.014	<0.002	<0.010	<0.010	<0.010	0.034	0.022	0.027
Total Kjeldahl Nitrogen as N	mg/L	0.10				1.00	5.7		1.20	2.10	0.62	0.50	0.46	0.60	0.60	0.40	0.57	0.46	0.39
Total Phosphorus as P	mg/L	0.01		0.05	0.18	0.12	0.4	0.08	0.03	0.35	0.006	< 0.005	<0.005	0.02	0.02	<0.01	0.014	0.026	< 0.005
Reactive Phosphorus	mg/L	0.01				0.066	< 0.004		<0.010	<0.010	< 0.004	< 0.004	< 0.004		0.01	<0.010	< 0.004	0.016	< 0.004
PH (lab)	pH Unit	0.01				7.43	8.20		4.43	5.06	6.40	6.40	3.80		7.06	7.47	7.60	7.40	7.50
Total Dissolved Solids (TDS)	mg/L	1			306	220	720	914	406	410	1742	2639.8	1815.7	360	324	336	406.69	425.45	589.6
Electrical Conductivity	uS/cm	1									2600	3940	2710				607	635	880
Suspended Solids (SS)	mg/L	1			21			31		50				11		7			
Total Hardness	mg/L	1			103			212										[]	
																		[]	
Major lons																			
Calcium	mg/L	1			24			32	18				60	43	42				47
Magnesium	mg/L	1			10			32	17				65	12	11				14
Sodium	mg/L	1			41			123	78				257	46	42			J	51
Potassium	mg/L	1			7			10	4				12	4	4				5.2
Bicarbonate as CaCO3	mg/L	1			75			<1	<1					48	47				
Total Alkalinity	mg/L	1			75			<1	<1					48	47				
Sulphate as SO4	mg/L	1			32			264	140				533	109	104				134
Chloride	mg/L	1			61.1			166	110				445	65.5	71				82
SAR																			
Calcium + Magnesium (meq/L)					2.02			4.23	2.30				8.34	3.13	3.00				3.50
Sodium (meq/L)					1.78			5.35	3.39				11.18	2.00	1.83				2.22
SAR= Na / Sqrt (Ca+ Mg) / 2)					1.01			1.45	1.07				2.04	1.25	1.22				1.32
SAR- Sodium Absorption Ratio					1.77			3.68	3.17				5.47	1.60	1.49				1.68
SAR Hazard Ranking					Low			Low	Low				Low	Low	Low				Low

Note:

SAR Hazard ranking based on Fetter, 1994. Low = 2 to 10, Med = 7 to 18, High= 11 to 26, V High= 26+ nr - no recommended NSW guidelines NA - Not Available TDS= EC*0.67 (approximate) calculation in italics (Data from Sept to Dec05)

Table 1D - Gerroa Monthly Groundwater Results (BH 1-6) Client Reference: Gerroa Bores

Client Reference:

NR = No Result - Dry

Notes	Report Number:	W05/0186	W05/0671	W05/1086-1	W05/1620	W05/2033-1	W05/2358	W05/2774	W05/3172	W05/3572-2	W05/3963	W05/4442	W05/4819			
Results:	Sample Receive	20/01/2005	24/02/2005	22/03/2005	27/04/2005	26/05/2005	22/06/2005	21/07/2005	22/08/2005	21/09/2005	21/10/2005	21/11/2005	20/12/2005			
Client Id		BH 1	BH 1	BH 1	BH1	BH 1	BH 1	BH 1	WM1	BH 1	BH 1	BH 1	BH 1	Minimum	Average	Maximim
Laboratory Id		W11016/002	W11511/002	W11945/002	W12413/002	W12828/002	W13143/002	W13573/002	W13973/003	W14389/002	W14768/002	W15175/002	W15693/002			
Conductivity (uS/cm)																
Method:APHA 2510 B	Units:uS/cm@25	1030	370	290	300	340	360	440	670	640	1010	590	470	290	531	1030
Groundwater level (RL)																
Method:	Units:m	0.48	1.83	1.73	1.53	1.73	1.33	1.73	1.12	1.39	1.05	1.33	1.21	0.48	1.39	1.83
рН																
Method:APHA 4500 H B	Units:pH units	6.60	5.70	6.00	6.50	5.80	5.80	5.70	6.0	5.80	6.00	6.90	6.1	5.70	6.08	6.90

Notes	Report Number:	W05/3172	W05/3572-2	W05/3963	W05/4442	W05/4819	W05/0671	W05/1086-1	W05/1620	W05/2033-1	W05/2358	W05/2774	W05/3172	W05/3572-2	W05/3963	W05/4442
Results:	Sample Receive	22/08/2005	21/09/2005	21/10/2005	21/11/2005	20/12/2005	24/02/2005	22/03/2005	27/04/2005	26/05/2005	22/06/2005	21/07/2005	22/08/2005	21/09/2005	21/10/2005	21/11/2005
Client Id		WM3A	BH 3A	BH 3A	BH3A	BH 3A	BH 4	BH 4	BH4	BH 4	BH 4	BH 4	WM4	BH 4	BH 4	BH4
Laboratory Id		W13973/007	W14389/013	W14768/013	W15175/003	W15693/013	W11511/004	W11945/004	W12413/004	W12828/004	W13143/004	W13573/004	W13973/005	W14389/004	W14768/004	W15175/004
Conductivity (uS/cm)																
Method:APHA 2510 B	Units:uS/cm@25	NR	NR	Dry	NR	Dry	NR	NR	NR	NR	NR	740	780	750	Dry	1080
Groundwater level (RL)																
Method:	Units:m	-	-	-	-	-	-	-	-	-	-	0.07	0.78	-0.33	-	0.57
рН																
Method:APHA 4500 H B	Units:pH units	-	-	-	-	-	-	-	-	-	-	6.4	6.8	6.9	-	7.90

Notes	Report Number:	W05/0671	W05/1086-1	W05/1620	W05/2033-1	W05/2358	W05/2774	W05/3572-2	W05/3963	W05/4442	W05/4819
Results:	Sample Receive	24/02/2005	22/03/2005	27/04/2005	26/05/2005	22/06/2005	21/07/2005	21/09/2005	21/10/2005	21/11/2005	20/12/2005
Client Id		BH 6	BH 6	BH6	BH 6	BH6	BH 6				
Laboratory Id		W11511/006	W11945/006	W12413/006	W12828/006	W13143/006	W13573/006	W14389/006	W14768/006	W15175/006	W15693/006
Conductivity (uS/cm)											
Method:APHA 2510 B	Units:uS/cm@25	NR	NR	NR	NR	NR	NR	NR	Lost	NR	NR
Groundwater level (RL)											
Method:	Units:m	-	-	-	-	-	-	-	-	-	-
pH											
Method:APHA 4500 H B	Units:pH units	-	-	-	-	-	-	-	-	-	-

Table 1D - Gerroa Mon

Client Reference: Gerroa Bor

NR = No Result - Dry

Notes	Report Number:	W05/3172	W05/3572-2	W05/3963	W05/4442	W05/0671	W05/1086-1	W05/1620	W05/2033-1	W05/2358	W05/2774	W05/3572-2	W05/3963	W05/4442	W05/4819
Results:	Sample Receive	22/08/2005	21/09/2005	21/10/2005	21/11/2005	24/02/2005	22/03/2005	27/04/2005	26/05/2005	22/06/2005	21/07/2005	21/09/2005	21/10/2005	21/11/2005	20/12/2005
Client Id		WM1A	BH 1A	BH 1A	BH 1A	BH 2	BH 2	BH2	BH 2	BH 2	BH 2	BH 2A	BH 2A	BH 2A	BH 2A
Laboratory Id		W13973/001	W14389/003	W14768/003	W15175/013	W11511/003	W11945/003	W12413/003	W12828/003	W13143/003	W13573/003	W14389/012	W14768/012	W15175/012	W15693/012
Conductivity (uS/cm)															
Method:APHA 2510 B	Units:uS/cm@25	NR	NR	Dry	NR	Dry	NR	Dry							
Groundwater level (RL)															
Method:	Units:m	-	-	-	-	-	-	-	-	-	-	-	-	-	-
pН															
Method:APHA 4500 H B	Units:pH units	-	-	-	-	-	-	-	-	-	-	-	-	-	-

Notes	Report Number:	W05/4819				W05/0671	W05/1086-1	W05/1620	W05/2033-1	W05/2358	W05/2774	W05/3172	W05/3572-2	W05/3963	W05/4442	W05/4819
Results:	Sample Receive	20/12/2005				24/02/2005	22/03/2005	27/04/2005	26/05/2005	22/06/2005	21/07/2005	22/08/2005	21/09/2005	21/10/2005	21/11/2005	20/12/2005
Client Id		BH 4	Minimum	Average	Maximim	BH 5	BH 5	BH5	BH 5	BH 5	BH 5	WM5	BH 5	BH 5	BH5	BH 5
Laboratory Id		W15693/004				W11511/005	W11945/005	W12413/005	W12828/005	W13143/005	W13573/005	W13973/006	W14389/005	W14768/005	W15175/005	W15693/005
Conductivity (uS/cm)																
Method:APHA 2510 B	Units:uS/cm@25	1110	740	892	1110	NR	NR	NR	NR	NR	100	NR	NR	Dry	NR	Dry
Groundwater level (RL)																
Method:	Units:m	-0.43	-0.43	0.13	0.57	-	-	-	-	-	0.23	-	-	-	-	-
pН																
Method:APHA 4500 H B	Units:pH units	7.2	6.4	7.04	7.90	-	-	-	-	-	5.0	-	-		-	-

Notes	Report Number:
Results:	Sample Receive
Client Id	
Laboratory Id	
Conductivity (uS/cm)	
Method:APHA 2510 B	Units:uS/cm@25
Groundwater level (RL)	
Method:	Units:m
pН	
Method:APHA 4500 H B	Units:pH units

Water Body	Lab Sample ID	Field Sample ID	Date	Decription	рН	Redox (mV)	EC (uS/cm)	TDS (mg/L)- #	DO (%)	Temp	Comments
Main Dredge Pond	M.D.Pond	FC-13	24/01/2005	Clear water	6.19	222	503	337	65	27.5	Groundwater
	M.D.Pond	FC-13	27/01/2005	Clear water	6.15	265	562	377	70	24.9	
	M.D.Pond	FC-13	2/06/2005	Clear water	6.35	190	655	439	80	8.3	(EC=1614m at 1.2m, pH 7.47)
	M.D.Pond	FC-13	3/08/2005	Clear water	6.3	185	528	354	65	17.2	dry period, low water level
	M.D.Pond	FC-13	21/10/2005	Clear water	7.73	92	298	200	95	20.92	Enviromanagers
	M.D.Pond	FC-13	21/11/2005	Clear water	7.58	96	617	413	100	23.32	Enviromanagers
	M.D.Pond	FC-13	20/12/2005	Clear water	7.86	82	647	433	99	22.83	Enviromanagers
South Dredge Pond	S.D Pond	FC-14	27/01/2005	Clear, >1m depth, no visible flow, vegetated	6.49	142	237	159	39	24.5	Vegetated, collects runoff?
	S.D Pond	FC-14	31/01/2005	Clear, >1m depth, no visible flow, vegetated	5.58	311	222	149	98	30	Vegetated, collects runoff?
GW Drain-1		FC-23	24/01/2005	clear, no visible flow, >1.5m depth 5m wide,	5.17	233	769	515	20	23.9	water level ~0.5m bgl
		FC-23	27/01/2005	clear, no visible flow, >1.5m depth 5m wide,	5.8	337	747	500	31	26.1	water level ~0.5m bgl
		FC-23	2/06/2005	clear, no visible flow, >1.5m depth 5m wide,	6.38	136	1164	780	47	14.1	water level ~0.5m bgl
		FC-27	3/08/2005	clear, no visible flow, >1.5m depth 5m wide,	6.64	-8	764	512	75	13.6	dry weather, low drain level
		FC-23	21/10/2005		7.95	73	2437	1633	100	21.24	Enviromanagers
		FC-23	21/11/2005		7.5	97	1247	835	100	21.44	Enviromanagers
		FC-23	20/12/2005		7.06	113	900	603	88	21.69	Enviromanagers
GW Drain-2		FC-18	24/01/2005	clear, no visible flow, >1.5m depth 5m wide,	6.31	158	1505	1008	50	25.3	water level ~0.5m bgl
		FC-18	27/01/2005	clear, no visible flow, >1.5m depth 5m wide,	6.3	278	1546	1036	35	26	water level ~0.5m bgl
		FC-18	3/02/2005	clear, no visible flow, >1.5m depth 5m wide,	6.19	103	1104	740	40	25	water level ~0.2m bgl
		FC-18	2/06/2005	clear, no visible flow, >1.5m depth 5m wide,	6.38	134	946	634	55	14.3	water level ~0.5m bgl
		FC-18	3/08/2005	clear, no visible flow, >1.5m depth 5m wide,	5.9	107	4070	2727	60	14.7	dry weather, low drain level
		FC-18	31/01/2005	clear, no visible flow, >1.5m depth 5m wide,	5.23	227	1534	1028	65	28.4	wet weather
	GW Drain-2	FC-19	24/01/2005	clear, no visible flow, >1.5m depth 5m wide,	6.64	162	1579	1058	55	24.1	
	GW Drain-2	FC-19	4/02/2005	clear, no visible flow, >1.5m depth 5m wide,	6.29	155	967	648	37	19.3	clear water
		FC-8	28/01/2005	shallow ditch near trees	3.62	353	3090	2070	60	29	drainage ditch, no flow
		FC-8	3/02/2005	shallow ditch near trees	4.6	14	323	216	32	27	wet weather
		FC-9	28/01/2005	shallow ditch near trees	3.33	453	2590	1735	45	33	Heavy vegetation
		FC-9	31/01/2005	shallow ditch near trees	3.34	227	1776	1190	37	31	Heavy vegetation
		FC-9	3/02/2005	shallow ditch near trees	4.73	117	288	193	47	23	wet weather
		FC-15	3/02/2005	clear, no visible flow, >1.5m depth,5m wide.	5.99	100	865	580	33	22.8	water level ~0.2m bgl
	GW Drain-2	FC-18	21/10/2005		7.85	99	7857	5264	100	20.61	Enviromanagers
	GW Drain-2	FC-18	21/11/2005		6.89	28	6574	4405	100	21.41	Enviromanagers
	GW Drain-2	FC-18	20/12/2005		7.26	118	4821	3230	85	21.69	Enviromanagers
GW Drain-3	GW Drain-3	FC-16	24/01/2005	clear, no visible flow, >1.5m depth,5m wide.	6.14	159	1320	884	60	24.6	water level ~0.5m bgl
	GW Drain-3	FC-16	2/06/2005	clear, no visible flow, >1.5m depth,5m wide.	5.02	125	1137	762	60	14.7	water level ~0.5m bgl
	GW Drain-3	FC-16	3/08/2005	clear, no visible flow, >1.5m depth,5m wide.	3.82	156	4020	2693	70	14.8	dry weather, low drain level
	GW Drain-3	FC-16	21/10/2005		8.3	94	7875	5276	100	21	Enviromanagers
	GW Drain-3	FC-16	21/11/2005		6.46	46	7976	5344	100	22.04	Enviromanagers
	GW Drain-3	FC-16	20/12/2005		7.22	102	5871	3934	90	22.87	Enviromanagers

Water Body	Lab Sample ID	Field Sample ID	Date	Decription	рН	Redox (mV)	EC (uS/cm)	TDS (mg/L)- #	DO (%)	Temp	Comments
GW Drain-4		FC-20	24/01/2005	Shallow <0.2m dish drain, stagnant water	3.58	339	1795	1203	50	26.1	acid waters
		FC-20	27/01/2005	Shallow <0.2m dish drain, stagnant water	3.36	475	1131	758	45	30	acid waters, rusty on bank
		FC-20	2/06/2005	Shallow <0.2m dish drain, stagnant water	4.17	310	1438	963	73	17.37	acidic, rusty on bank
	GW Drain-4	FC-10	28/01/2005	shallow drain, no flow	3.03	381	4680	3136	15	35.1	drainage ditch, no flow
		FC-11	28/01/2005	shallow drain, no flow	3.83	390	1227	822	50	34	drainage ditch, no flow
	GW Drain-4	FC-10	21/10/2005		7.7	108	12310	8248	100	21.27	Enviromanagers
	GW Drain-4	FC-10	21/11/2005		3.76	362	923	618	100	23.79	Enviromanagers
	GW Drain-4	FC-10	20/12/2005	dry							
GW Drain-5	GW Drain-5	FC-26	27/01/2005	clear, no visible flow, >1.5m depth,5m wide.	6.79	356	2117	1418	50	28	clear water
	GW Drain-5	FC-26	3/02/2005	clear, no visible flow, >1.5m depth,5m wide.full	6.39	75	1563	1047	46	21.8	clear water, wet weather flow, full drain
	GW Drain-5	FC-26	2/06/2005	clear, no visible flow, >1.5m depth,5m wide.full	7.32	173	1554	1041	51	11.4	clear water
	GW Drain-5	FC-26	3/08/2005	clear, no visible flow, >1.5m depth,5m wide	6.63	192	3250	2178	60	14.6	dry weather, low water level
		FC-7	28/01/2005	south end of drain 5	6.23	99	505	338	100	24	groundwater, deep drain, clear water
		FC-7	31/01/2005	south end of drain 5	5.41	261	863	578	50	26	groundwater, deep drain, clear water
		FC-7	3/02/2005	south end of drain 5	5.85	89	1082	725	36	19.6	groundwater, deep drain, clear water
	GW Drain-5	FC-26	21/10/2005		7.57	129	8242	5522	96	21.33	Enviromanagers
	GW Drain-5	FC-26	21/11/2005		7.54	116	7289	4884	98	22.35	Enviromanagers
	GW Drain-5	FC-26	20/12/2005		7.54	109	5331	3572	76	23.09	Enviromanagers
Drain-6	GWDrain-6		3/08/2005	slight flow (1L/min)	6.93	103	2120	1420	75	14.3	dry weather, low water level
	GWDrain-6		2/06/2005	slight flow (1L/min)	7.11	156	1173	786	60	9.4	slow flow
	GWDrain-6		21/10/2005		7.71	109	2394	1604	100	21.68	Enviromanagers
	GWDrain-6		21/11/2005		7.86	102	1949	1306	100	21.21	Enviromanagers
	GWDrain-6		20/12/2005		7.75	100	2131	1428	72	19.26	Enviromanagers
SW Drain	SW Drain	FC-29	27/01/2005	Shallow drain (<1m), flowing (~1L/sec), heavy vegetation, cow dung	6.48	171	256	172	10	23.3	cow dung, odour, turbid, gw seepage
	SW Drain	FC-29	2/06/2005	cow dung	6.83	127	312	209	47	14.7	cow dung, odour, turbid, gw seepage
	SW Drain	FC-29	3/08/2005	cow dung	7.11	5	340	228	70	15.3	dry weather, low drain levels- stagnant
	SW Drain	FC-29	21/10/2005		7.65	87	465	312	92	20.66	Enviromanagers
	SW Drain	FC-29	21/11/2005		7.98	80	389	261	100	20.91	Enviromanagers
	SW Drain	FC-29	20/12/2005		7.6	82	373	250	81	17.36	Enviromanagers
W Drain	W Drain	FC-28	27/01/2005	3m wide, approx 1m deep, still water	7.08	185	417	279	38	22.8	slight turbid, brown, vegetated drain
	W Drain	FC-28	2/06/2005	3m wide, approx 1m deep, still water	7.49	120	448	300	70	17.4	slight turbid, brown, vegetated drain
	W Drain	FC-28	3/08/2005	3m wide, approx 1m deep, still water	7.4	11	440	295	75	15.8	slight turbid, brown, vegetated, stagnant
	W Drain	FC-28	21/10/2005	dry							
	W Drain	FC-28	21/11/2005		8.14	79	689	462	154	19.45	Enviromanagers
	W Drain	FC-28	20/12/2005		7.56	87	539	361	59	15.56	Enviromanagers

Dredge Pond, Foys Swamp and Blue Angle Creek

Water Body	Lab Sample ID	Field Sample ID	Date	Decription	рН	Redox (mV)	EC (uS/cm)	TDS (mg/L)- #	DO (%)	Temp	Comments
NW Drain	NW Drain	FC-3	27/01/2005	3m wide, approx 1m deep, still water, heavy vegetation	4.1	188	1103	739	16	24.2	still water, vegatation in drain, slight turbid
		FC-1	27/01/2005	shallow drain, no flow	3.54	355	1114	746	26	25.3	acid waters
		FC-2	27/01/2005	deep drain, no flow	3.22	462	1642	1100	40	25.8	acid waters
	NW Drain	FC-3	27/01/2005	deep drain, no flow	3.4	463	1390	931	28	26.5	acid waters
		FC-4	27/01/2005	deep drain, no flow	3.16	470	2099	1406	30	27.3	acid waters
		FC-5	27/01/2005	shallow drain, no flow	3.33	406	4810	3223	41	30	acid waters
	NW Drain	FC-3	2/06/2005	deep drain, no flow	4.81	258	720	482	47	12.7	Heavy vegetation
		FC-4	3/08/2005	deep drain, no flow	4.82	74	1918	1285	73	13.6	dry weather, low drain levels
	NW Drain	FC-3	3/08/2005	deep drain, no flow	4.98	81	672	450	65	14.6	dry weather, low drain levels
	NW Drain	FC-3	21/10/2005		6.91	128	3214	2153	67	21.32	Enviromanagers
	NW Drain	FC-3	21/11/2005		4.11	337	2079	1393	100	22.6	Enviromanagers
	NW Drain	FC-3	20/12/2005		6.56	90	1636	1096	79	21.22	Enviromanagers
Main Drain (up stream)	MDrain-1	FC-17	24/01/2005	clear, no visible flow, >1.5m depth,5m wide.	5.73	158	781	523	35	25.3	water level ~0.5m bgl
	MDrain-1	FC-17	27/01/2005	clear, no visible flow, >1.5m depth,5m wide.	5.42	276	805	539	34	26.6	water level ~0.5m bgl
	MDrain-1	FC-17	31/01/2005	clear, no visible flow, >1.5m depth 5m wide,	5.35	130	1034	693	61	28	wet weather
	MDrain-1	FC-17	2/06/2005	clear, no visible flow, >1.5m depth,5m wide.	6.38	136	1164	780	47	12.6	water level ~0.5m bgl
	MDrain-1	FC-17	3/08/2005	clear, no visible flow, >1.5m depth,5m wide.	6.3	26	2101	1408	33	13.2	dry weather, low drain levels
	MDrain-1	FC-17	21/10/2005		7.27	97	7293	4886	62	21.55	Enviromanagers
	MDrain-1	FC-17	21/11/2005		7.28	109	2506	1679	111	21.22	Enviromanagers
	MDrain-1	FC-17	20/12/2005		7.24	115	3192	2139	77	21.41	Enviromanagers
Main Drain (dn stream)	MDrain-2	FC-32	27/01/2005	clear, no visible flow, >1.5m depth,4m wide.	6.52	222	1309	877	24	27.8	water level ~0.5m bgl
	MDrain-2	FC-32	4/02/2005	shallow drain, no flow	5.5	180	960	643	35	19.3	clear water
	MDrain-2	FC-32	2/06/2005	moderately full	7.06	166	1194	800	42	10.9	clear water, drain almost full
	MDrain-2	FC-32	3/08/2005	clear water, no visible flow, >1.5m depth,4m wide.	6.98	112	2330	1561	63	14.1	clear water. Dry weather - low level
	MDrain-2	FC-32	21/10/2005		7.34	130	6827	4574	70	22.97	Enviromanagers
	MDrain-2	FC-32	21/11/2005		6.89	28	6574	4405	125	21.41	Enviromanagers
	MDrain-2	FC-32	20/12/2005		7.43	107	3485	2335	74	23.5	Enviromanagers
Large Dam	LD-2/ M Dam	FC-30	27/01/2005	Full dam next to Beach Rd, 300 MG capacity	7.01	160	154	103	50	24.8	slight turbid, brown.
-	M Dam	FC-30	2/06/2005	Full dam next to Beach Rd, 300 MG capacity	8.42	162	177	119	95	17.2	slight turbid, brown.
	LD-2/ M Dam	FC-30	3/08/2005	Full dam next to Beach Rd, 300 MG capacity	6.12	78	170	114	80	14.5	slight turbid, brown.
	M Dam	FC-30	21/10/2005		8.32	62	294	197	100	20.09	Enviromanagers
	M Dam	FC-30	21/11/2005		6.77	95	186	125	100	22.55	Enviromanagers
	M Dam	FC-30	20/12/2005		7.73	80	194	130	100	21.85	Enviromanagers
Small Dams		FC-22	24/01/2005	clear, small, vegetated, <0.7m deep	6.35	199	87	58	50	27	very shallow, no flow.
		EC-21	24/01/2005	Clear water, dam next to Beach Rd 10 MG capacity	6.4	200	232	155	65	25.8	-0.5 mbg 50m by 50m 3.5m deep
		FC-24	24/01/2005	Clear water, dam downhill of large dom	6.95	170	159	107	85	26.1	=0.5 mbg, contrues leakage from large dam
		FC-31	24/01/2005	Clear water, small snallow carm next to Beach Ro, vegetated	6.15	216	143	96	55	25.3	Heavy vegetation
Blue Angle Creek (dn)	BA Creek	FC-25	31/01/2005	5-10m wide, next to flood gates	4.79	275	1351	905	26	25.8	slightly turbid, brown, wet weather
	BA Creek	FC-25	3/02/2005	5-10m wide, next to flood gates- moderate flow	6.69	392	392	263	50	26	slight-mod turbid, brown- wet weather flows
	BA Creek	FC-25	2/06/2005	5-10m wide, next to flood gates- moderate flow	6.7	174	21820	14619	43	15.5	high tide
	BA Crook	EC-25	3/08/2005	5-10m wide, next to flood gates- low flow	6.58	109	6320	4234	33	16.2	Low tide, mouth open, slight turbidity

Dredge Pond, Foys Swamp and Blue Angle Creek

Water Body	Lab Sample ID	Field Sample ID	Date	Decription	pН	Redox (mV)	EC (uS/cm)	TDS (mg/L)- #	DO (%)	Temp	Comments
	BA Creek	FC-25	21/10/2005		7.28	138	9981	6687	81	22.69	Enviromanagers
	BA Creek	FC-25	21/11/2005		7.15	85	5229	3503	92	21.35	Enviromanagers
	BA Creek	FC-25	20/12/2005		7.32	85	10460	7008	73	17.48	Enviromanagers
Notes:				Min	3.03	5	87	58	10	8	
E2W Field Equipment Calibrated:	Field Kit 90 FLMVSA (EnviroEquip Pty Ltd)		Max	8.42	475	21820	14619	154	35	
mbal= metres below around level				Average	6.24	171	2353	1577	65	22	

Enviromanagers conducted sampling from September 2005 onwards

Table 5- Groundwater Field Chemical Parameters Cleary Bros - Beach Road, Berry.

Well ID	Date	Time	SWL (m bgl)	Stick up (m)	BOH (m bgl)	Volume Purged (L)	рН	Redox (mV)	EC (uS/cm)	TDS (mg/L)- #	DO (%)	Comments
New Wells	s (Jan 05)											period of wet weather and bodgy ground
												slight turbidity, brown, rapid recovery (high K),
GW-A	3/02/2005	8.19 am	0.00	0.70	2.20	50	6.02	37	140	94	30	some sw ingress
	31/01/2005	1.30pm	0.10	0.70		1	6.82	95	337	226	4	trace H2S odour (field chem only)
	8/02/2005	9.30 am	0.30	0.70								excavation area
	28/04/2005	9.30 am	0.72	0.70	2.30							no field chem (lab analyses)
	2/06/2005	9.15 am	0.24	0.70	2.30							
	3/08/2005	12.15 pm	0.58	0.70	0.92	2						insufficient sample- slow recovery when purged
	21/11/2005	1pm	0.91	0.70			6.66	-29	555	372	67.4	Enviromanagers
	20/12/2005	2pm	0.96	0.70								Enviromanagers
GW-B	3/02/2005	8.19 pm	0.00	0.77	2.20	15	4.21	143	2228	1493	49	clear water, moderate recovery, some sw ingress
	31/01/2005	2.30pm	0.00	0.77		1	4.17	302	3460	2318	15	slight turbidity, brown
	15/02/2005	2.00 pm	0.70	0.77								water level measured during dry period
	2/06/2005	11.30am	0.60	0.77								
	3/08/2005	2.20pm	0.48	0.77	1.98	2	5.31	-36	3310	2218	10	turbid, H2S odour
	21/11/2005	11am	0.63	0.77			4.27	30	2806	1880	65	Enviromanagers
	20/12/2005	2.50pm	0.63	0.77								Enviromanagers
GW-C	4/02/2005	8.30am	0.00	0.73	2.20	8	3.75	200	7540	5052	28	slightly turbid- brown, slow gw recovery
	15/02/2005	2.00 pm	0.71	0.73								water level measured during dry period
	28/04/2005	11.30am	0.49	0.73	2.20							no field chem (lab analyses)
	2/06/2005	11.40am	0.48	0.73								
	3/08/2005	2.30pm	0.58	0.73	2.58	2	5.5	-108	7690	5152	9.5	turbid, H2S odour
	21/11/2005	1.30am	0.58	0.73			4.55	28	6114	4096	56.7	Enviromanagers
	20/12/2005	3pm	0.71	0.73								Enviromanagers
GW-D	4/02/2005	9.00am	0.00	0.50	2.20	10	3.81	232	7510	5032	14	test
	15/02/2005	2.15 pm	0.77	0.50								water level measured during dry period
	2/06/2005	3.20pm	0.49	0.50								
	3/08/2005	1.40pm	0.60	0.50	2.13	2	5.81	-23	3510	2352	8	turbid water, slow recovery
	21/11/2005	11am	0.76	0.50			3.91	103	3267	2189	40.7	Enviromanagers
	20/12/2005	3.30pm	0.93	0.50								Enviromanagers
GW-E	4/02/2005	9.40am	1.40	0.71	2.03	1	4.27	193	2560	1715	30	mod turbid - brown, v slow gw recovery
	15/02/2005	3.00pm	1.41	0.71								higher ground
	2/06/2005	3.30pm	0.58	0.71								
	3/08/2005	2.00pm	0.63	0.71	1.97	2	5.62	-33	3730	2499	10	turbid water, slow recovery
	21/11/2005	11.20am	0.91	0.71			4.67	63	4126	2764	51.4	Enviromanagers
	20/12/2005	4pm	1.04	0.71								Enviromanagers
GW-F	3/08/2005	12.30pm	0.42	1.15	1.77	2	6.14	122	502	336	33.3	turbid water, slow recovery
-	28/04/2005	9.30am	0.58	1.15	1.77							dry weather
	2/06/2005	10.10am	0.21	1.15								
	21/11/2005	1.10pm	0.68	1.15			6.4	5	545	365	77.3	Enviromanagers
	20/12/2005	4.30pm	0.81	1.15								Enviromanagers
GW-G	28/04/2005	9.45 am	0.62	1.15	1.32							dry weather
	2/06/2005	10am	0.18	1 15								
	3/08/2005	1.00pm	0.55	1 15	1.32	2	6.86	89	630	422	25	turbid water slow recovery
	21/11/2005	4.48	0.77	1 15		-	6.98	-86	400	268	36.9	Enviromanagers
	20/12/2005	1nm	0.88	1.15			0.00	00	400	200	00.0	Enviromanagers
	20/12/2000	ipin	0.00	1.10								Environnanagoro
Existing Wells												
MW-2R	28/04/2005	12.30pm	0.60	0.66	1.22	-						dry weather
	3/08/2005	11.45am	0.70	0.66	1.22	2	NA					insufficient sample- slow recovery when purged
	21/11/2005	-	dry	0.66								
	20/12/2005	2pm	dry	0.66								Enviromanagers
MW-1#	3/02/2005	3.00pm	0.61	0.23	3.85	10	6.27	74	452	303	19.9	Very turbid, grey (frogs inside well)

Notes: E2W - Field parameters (ph, EC etc) noted are at end of purging and start of sampling. E2W Field Equipment Calibrated: Field Kit 90 FLMVSA (EnviroEquip) SWL= standing water level BOH= bottom of bore mbgl= metres below ground level TDS = EC*0.67 (approximate)

Monthly Water Monitoring Results - GW and SW atia

					_				_									-	_
	ameter	-9	24/2003	27/2003	19/2003	31/2003	30/2004	26/2004	31/2004	29/2004	27/2004	28/2004	28/2004	27/2004	27/2004	27/2004	26/2004	29/2004	20/2005
(D)	Par	5	09/2	10/2	11/	120	014	02/2	03/5	04/2	05/2	06/2	910	08/2	2/60	10/2	11/2	12/2	01/2
	CLSO4 Rate	mg/I			10		-				23	- com		-			5.2		-
	Conductivity	uS/cm@25 deg C	898	1113	970	615	534	489	382	460	365	323	278	396	479	430	710	780	1030
	Groundwater Level	m	0.51	0.33	-0.06	1.5		1.93	1.63	1.65	1.69	1.53	1.65	1.73	1.7	1.94	1.48	1.43	0.48
1414.475	pH	pH units	6.3	6.6	5.4	8.4	5.9	5.8	6.2	5.4	6.3	6.1	5.7	6.2	5.8	6.5	6.7	6.5	6,6
WM2	CI-SO4 Ratio	mg/l			3.3	NR	NR	NR	NR	NR	NR	NR	NR	NR	NR	NR	NR	NR	NR
	Conductivity	uS/cm@25 dea C	466	530	465	1.4.4		1.012						- rac	1	1000	Sec.	1.000	
	Groundwater Level	m	0.29	0.17	-0.03	-		-			-	-							-
	pH	pH units	7	7	7.4	-	-		-							-	-	-	-
WM4		1. 1. 2. 2. 2. 2. 2. 2. 2. 2. 2. 2. 2. 2. 2.	1																
	CLSO4 Ratio	mg/l	NR	NR	NR	NR	NR	NR	NR	NR	NR	NR	NR	NR	NR	NR	NR	NR	NR
	Conductivity	uS/cm@25 deg C				-	-			1.00	1	1	-						
	Groundwater Level	m objunite	-	-	-	-	-	-	-		-	-	-		-	1	-	-	-
WM5	pr	pri units	-	-		-		-		1	-	-	-	-				-	
THIS	C:504 Ratio	mg/l	NR	NR	NR	NR	NR	NR	NR	NR	NR	NR	NR	NR	NR	NR	NR	NR	NR
	Conductivity	uS/cm@25 deg C				-						1 1							
	Groundwater Level	m			-									-				-	
	pH	pH units												1					
WM6	0.0010-00	mall	ND	NID	ND	ND	ND	NID	ND	AUD.	AUD.	NID	ND	NID	NID	NO	NID	NID	AID
	Criston Ratio	Ing/I	INPS	INPL	INPA	INK	INR	NR	INIK	NR	INPR	INK	INPO	NP	INPS	NR	INR	NR	INR
	Conductivity	us/cill@25 deg C	-		-	-	-	-			-		-		-		-		
	GIOLER Water Level	nH units		-	-	-													-
WM7	pri	pri unito				-		-			-		-	-	-			-	-
	CLSO4 Ratio	mg/l			4.8						3.8	-			-	-	3		
	Conductivity	US/cm@25 deg C	325	293	245	257	324	290	285	248	268	292	262	339	336	180	210	190	160
	Groundwater Level	m	-0.39	-0.31	-0.34	-0.23	-0.37	-0.41	-0.51	0.09	-0.29	-0.26	-0.28	-0.41	-0.41	0.09	-0.21	1.1	-0.46
	opti.	pH units	5.5	5.7	5.8	6.1	5,9	5.7	5,6	6	5.8	5.6	5.6	5.4	5.2	5.8	5.4	5.5	5.7
WM9			-			-	_	-			0.00		-	-	-	-		-	
	CI:SO4 Ratio	mg/l	400	470	1.4	170.0	224	200	705	200	0.63	100	1 100		770	110	1.1	270	000
	Conductivity	us/cm@25.00g.C	162	0.88	100	1/0.9	0.05	336	125	309	219	188	192	214	2/6	0.70	350	3/0	860
and the second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second s	GIOCINOWING COVO	ation Ha	57	-0.00	-0.92	6.1	-0.95 B.A	6.2	7	6.5	6.1	61	82	5.5	-0.95	6.7	6.75	-0.8 E 1	-1.10 6.4
WM11	60.	Produce	5.7	5.5		0.1	0,4			0.5	0.1	1		3.4	2.4	0.4	0.5	0.1	0.4
COLUMN 1	CLSO4 Ratio	mg/l			5.8	1					4.4	1					3.6		
	Conductivity	US/cm(g)25 deg C	1358	1178	798	660	561	912	NR	688	1325	804	1231	1202	2110	1540	840	970	1110
	Groundwater Level	m	-1.08	-0.88	-0.87	-1.66	-1.46	-1.38		-1.52	-1.23	-1.48	-1.46	-1.55	-1.46	-1.38	+1.38	+0.98	-1.53
	pH	pH units	5	5.3	5.1	6	5.5	5.4		5.4	5	5	4	5.1	49	48	59	В	64
WM12			-	-		-	-	_	-	-	2.0						1.0	-	-
	CI.SO4 Ratio	mg/l	400	404	5.5	0700	2000	040	247	240	3.8	0.90	045	205	004	240	1.3	000	420
	Conductivity	us/cm@25 deg C	400	404	434	2/00	3080	312	34/	240	234	3/3	612	0.20	907	0.44	0.0	020	430
	Croundwillin Levis	ali unite	82	-0.00	6.2	B	6.4	6.1	6.2	61	87	4.20	34	3.0	33	6.0	30	5.4	54
WM14	- pro	pri units	0.4		0.2		0.4	0.1	0.2	0.1	0.2	4.5	34	3/3	2.0	0.4	3.0	2.4	2.4
August.	CLSO4 Ratio	mg/l	NR	NR	NR	NR	NR	NR	NR	NR	NR	NR	NR.	NR	NR	NR	NR	NB	NR
	Conductivity	uS/cm@25 dog C									_	1.1							
	Groundwater Level	m		-							1.1.1.1								
2.2.2.1	pH	pH units									1				-			-	_
Ex Works	D.CO.I.D.J.	21.01	-	-	0.04	-	-	-			NID	-					0.71		
	CLSOM Rate	mg/i	1220	C+C	0.84	646	054	ere	660	602	NR	6.74	646	770	003	610	530	630	660
2	Con actuator Loval	m m	1000	013		013	0.54	000	000	005		524	242	114	304	310		0.00	500
	pH	pH units	81	B.1	7.5	8.4	8.4	7.5	7.8	7.5	· · · · ·	7.4	7.2	73	7.6	7.8	7.5	7.4	7.8
Blue Angle Creek		The lack states																	
	CI:SO4 Rato	mg/l			6.3	in the		1.00		1.1.1	6.4	1	1		1000		5.8		1
	Conductivity	uS/cm@25 deg C	3670	3910	3980	10830	10830	1538	24600	5660	4520	14950	7890	16540	36400	750	10010	20400	20000
	Groundwater Level	m	-		-		-	~	-	-	-	-	1		-	-	~		
South Dood	pH	pH units	6.8	6.7	7.1	7.1	1.2	6.4	6,7	6.6	5.9	6.8	63	5.4	6.6	3.5	/	6.9	6.9
South Mead	CISO4 Rate	ma/l	NR	NR	0.86	NR	NR	NR	NR	NR	NR	-	-	-	-	-			
	Conductivity	uS/cm@25 dea C			558		1415	1415	195			-		1		1			-
	Groundwater Level	m			-														
	pH	pH units			7.5	1													

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CLEARY BROS(BOMBO) PTY LTD Southern Extraction Area, Gerroa Groundwater Monitoring

Ref:123r6lgerwm.wkd

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Table B.2 - 1

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14-Mar-34	492030+->	>	5008		74045		23145 55045	563.49	220US			163445	462uS	403uS	782uS	37.7mS		433uS	322µS
ATE	אר אפ≺⊳¥	>w:_		111	0.47		0.12	11	0.03			41.0	0.26	-0.11	(-0.33)			7 G	0.16
		REMARKS				 .			-										
		Н	06		7.8	4 F	8.4		8.0		,		7.3	7.1	9.9	6.2		7.6	7.6
4-Nov-03	002030F->	>	661u3	106645	448US	5 I I I	6064S	12BuS	142uS		0.000	C 1077	207uS	16245	473uS	17.2mS		87645	480uS
DATE 2	}≪≻4K 787			1.63	0.36	14	0.32	0.25	0,11		67.0	0.25	0.42	0.07	(-0.33)			0.06	0.70
		REMARKS																	
		Hq	8.2	77	7.9	1.5	7.6	8.1	8.3		7.4		7.1	7.1	7.2			9.1	1.4
07-Oct-33	0 020 30+->-	- 1- 3-	482uS	1230uS	410uS	18045	490uS	126uS	91uS		16645		211uS	161uS	166uS	7.04mS	ATTLE A	\$97uS	264uS
DATE	¥≺⊬¤z ⊔u>	ي. ت		1.88	10.0	0.37	0.60	0.48	0.28		0.65	0.45	0.70	0.29	160.0-1		0.41	0.38	0.77
		REMARKS			70 800055												:		
		На	6. 81	6.7		8.2	7.8	8 2	8.2		7.8		6. 9. 1	(a) ((a) (7.0	7.6	7.8
03-Sep-93	002020>-		614uS	23043		20945	617uS	125uS	136uS		485uS		193uS	12805	01701	SUI7 77	1.36mS	773uS	248uS
DAIE	יכמי אח≺≥≹	4.4 1		0.78	99.0	0.20	0.60	0.38	0.38		0.34	0.02	0.36	47.0	162.01		0.41	0.46	0.27
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	-	Hu	1	9.1		2.5	0. 00	8.2	8.0										
20-11-20A	0Z030F->-			108845		266uS	46645	202uS	14645								_		
	×⊷wæ Jw>w	- -	0.00	S C		0.07	0.37	0.38	RZ-0									-	:
•	VIION	tr apra	9	20.7 9 H S	24	1.47	25.5	4.53	4./8 GLE P/T		1.69	2.34	2.00	0.92	E CREEK	6 151 E	3.40	6.10 2.44	E CREEK
	LOCA	LITHERN BY TH	LOCATION	-	[WM3	WM4	SHOW Show	BLUE AN	LOCATION	7MM7	MM8	WM8	WM11	BLUE ANGL	BAKEY	VM12	WM13	BLUE ANG

CLEARY BROS(BOMBO) PTY LTD SOUTHERN EXTRACTION AREA, GENROA GROUNDWATER MONITORING

Ref. 123r5/general wied

Davron Engineering Pty Ltd

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Table B.2 - 2

CLEARY BROS(BOMBO) PTY LTD SOUTHERN EXTRACTION AREA, GERROA OROUNDWATER MONITORING

Ref. 123-6 Gerwan. - Me

Davron Engineering Pty Ltd

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Table B.2 - 3

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	ph Remarks	8.0	6.7	7.7	6.4	6.7	7,6	6.8		6.6		6.7	6.4	6.6		6.0	8.9
26-Oct-34	00ID30F->-F>	607uS	1760uS	336uS	142uS	377uS	136uS	884uS		426uS		214uS	38845	40.1mS	· · · · · · · · · · · · · · · · · · ·	2.6mS	86045
DATE	עע ≩≪⊢шқ ⊐ш>∞⊐		0.92	0.56	0.03	0.15	0.17	0.05		0.16	Đ.12	0.10	- 121			0.18	0.31
	REMARKS																
	<u>स</u>	8.1	6.7	7.1	ê.6	8	7.0	6.7		6.3		6.4	5.2	9.9		9.9	0.7
13-Oct-94	×+-<-40000	Suce	1600uS	STOUS	262uS	406uS	13745	12745		663uS		226u\$	36705	48.9mS		3.29mS	926uS
DATE	אראכיתר אחילצ		0.95	0.67	-0.08	0.14	0.16	-0.05		0,16	0.05	0.07	(0 13)			0; 10;	0.27
	REMARKS							: :	-		•			-			
	Ŧ	7.6	6.8	8,	6.4	0.7	7.3	1.1		6.3		e, e	. 7 3	6.4		5.0	4. 4
21-Sep-54	UØ I QIUF->>	606uS	1330uS	1162uS	168uS	420uS	104uS	38uS		326uS		20645	46305	46mS		2.4ms	3004S
DATE	לאר אחלאל≹		1.21	0.70	0.01	0.14	0.21	0.06		0.27	6.0	0.12	(1 27)				0.29
	ATION	AC. AREA AHD	2.83	5.86	1.47	5.57	4.53	4.78	AF40	1.69	2.34	2.00	2.34	LE CREEK	jists.	3.40	6,10 2,10
	LOCA	LOCATION	WM1	WM2	WM3	WM4	SMW	WMG	BI UE AP	ZMM	WM8	BMW	WM10	BLUE ANG	BALEY	WM12	WM13

CLEARY BROS(BOMBO) PTY LTD SOUTHERN EXTRACTION AREA, GERROA GROUNDWATER MONITORING

Ref: 123r5\genwin.wk4

Davron Engineering Pty Ltd

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Table B.2 - 4

GROUNDWATER LEVELS & pH READINGS 12/1794 - 11/1/96

	_	H	7.3	6.7	1.3	1.7	7.6	7.0	E.	6.9	6.2	9.6		8.8	8.9	0'Z	0.7	6.2	8.6	7.1	7.0	6.3	55	6.0	6.0	6.3	8.9	8.5	en en	8.9	8.8
	Μ	9	222	567	224	258	220	210	388	382	256	280		ŧ	334	0.21	112	270	330	380	ş	305	362	745	8	430	697	870	330	£33	852
	5	×. K	0.16	0.67	0.81	0.57	9.64	0.33	+ 1 0	0.07	0,08	0.03	•	11.0	10	0, 16	9.6	0.71	0.55	0.35	61.0	0.77	0.73	0.88	0.62		0, L4	0.02	50 T		60.0
AND		H	1.3	2.0	14	6,9	6.7	1.0	6.4	7.0	8.9	7.7	•	7.3	-	7.2	7.0	6.5	*7 19	6.8	8,8	8 , 4	8	6,9	6.7	8. *	6.7	8.8	1	8.8	8.8
LS ISL	ĒΨ	8	1053	728	587	\$63	505	819	900	925	098	200		2.17	110	0.79	966	641	123	816	840	630	585	619	786	139	7 97	993	123		80.1
AILEY	5	N.L	0.03	6.6	0.42	0.23	9.14	1.0	0.29	0.27	16.0	0.21		0 22	8	5.5	131	147	52	61 (900	0.22	0.27	0.21	0,18	80	90'0	0.07	0.01	•	10'0
		H	7.1	3.6	7.B	- - -	6.9	65.6	+	8.9	6.0	*	2.9	7.1		6.9	6.7	-	8.2	6.7	8.7	8.8 0	2.2	3.7	9					8.7	6.2
	MI2	9	13	604	Ę	502	4ts	683	48	29	5 N	3.6	14	36	+	6.1	61.1	571	8	6.28	1.37	026	Ę	131	8	31	643	E		B	23
	3	×.⊢ V	0.14	\$	1.23	10	8	03	•	13	8	90'	1 B1 1	0.1	60		50	31	80	3.02	08	33	5	81	103	1 70.0	1	0.17 2		-	1 80'0
		Hq	6.7	8.5		6.1	5.3	5.2	1. J	5.2	3.1	8.4 C	4.8	B \$	6.3	8 1	2.2	4.9		2 O	9 9	4.5	4°8	6.4	1		5.7 0		3.9		3.5
	IΨ	9	782	435	350	367	\$10	573	463	267	88	230	620	917	1552		10	213	867	340	315	910	463	121	638	558	428	\$30	315	200	78t
	5	N.L.	0.39	82.0	0, 17	0.32	0.27	0.02	1,27	5.1	11	10,0	117	5	0.08	66.0	60.0	0.1	211	0.2	03	80 1	0.25	0.28	.08	22	138	0.03		0.28	.27
		Hd	7.5	7.3	7.7	1.2	•	•	•								•			 		, ,	•	•			•	•	•		ې ب
,	MI0	9	403	239	239	178	 	•		•	·	-	 ,			•	•							•	•				-		
	5	Ň		0.36	0.34	0, 18	•		•	•						•	•	,		•			•		·		•				
E PIT		μ	7.2	1.1	4	7.4	7.3	5,33	6. 10	6.4	5.7	9	99	6.3	9 [.] 6	6.7	6. 9	5.9	6.1	5.7	¥:	5.8	3.2	5.7	6'S	6.0	7.2	8.9	8	1.0	6, T
INCLU	ξΨλ	9	462	217	234	178	153	174	205	226	514	344	154	257	505	247	226	203	205	309	250	217	231	189	186	295	810	305	126	156	380
PINE /	>	N.L	0.25	0.57	0.7	0.52	0.61	0.32	0.12	0,07	- 0	0.01	0.22	0.54	0,01	4	0.13	0.65	0.53	10.0	0.25	0.77	57.0	0.78		12	0.21	0.02	0.43		0.3
		H	•					•			•						•			•		•	•	•			•	•	•	-+	•
	VM8	9		•	•	,	•	•	•								•	•	-	,			•	. •	•		•		•	-	•
	>	V.L	0.09	Q. 33	0.5	16.0	0,48	0.41	0.01	-0.05	0.13	0.24	0.19	0.21	0.21	0.05	0.36	0.44	0.35	0.17	•	0.49	,	0,42	,				•	•	•
		Ha	7.7	7.2	7.2	7.8	1.3	5.63	6.3	6.3	9 9	7.3	7.1	7.01	6.9	51 19	6.2	8,3	5.5	40 19		5,4	3,8	6.0	6.1	8.8	4	7.1	6.3	1.7	6.2
	VM7	8	1534	198	187	156	164	161	326	553	425	£65	889	532	624	475	189	139	195	249	320	191	195	108	258	218	225	293	225	275	234
		V.L	0.14	0.55	0.74	0,56	0.62	0.39	0.27	0.15	0.16	0.05	0.12	0.17	0.07	0.31	0, 18	0.66	0.58	0.43	0.36	0.68	0.82	0,86	0.7	0.53	0,26	0.15	0.31	0.19	0.25
		Н	6.7	6.7	8.2 2	7.8	8.2	6,51	7.1	6.7	8.8	7.5	6.7	6.7	7.0	7.4	6.7	6.9	6.7	7.5	6.7	5.8	6.5	6,7	6.9	6.3			,	•	•
	NM6	Ð	220	1.163		88	13	96	88	127	83.4	108	121	133	143	88	76	52	209	167	158	142	168	66	2.8	16			,		•
		W.L	-0,03	0, 16	0.33	0,19	0.20	0,03	0.06	-0.05	0.05	-0.03	0.05	0.03	0,06	0.22	0.06	0.30	0.15	0.10	0.03	0.25	0.3	0.28	0. I9	1.0	10.0	-0.07	-0,02	-0.03	-0.04
		Ηď	7.5	67 EB	4.8	8.4	<u>8</u> ,3	6,69	7.3	7.0	7.8	7.8	7.0	6.5	6.8	7.5	£.7	6.8	7.3	7.7	6.8	6.8	6.5	6.9	6.8	6.6		,		•	•
	VM5	8	252	211	* 	Ξ	96	86	104	137	135	184	176	162	490	185	17	Ŧ	132	153	133	122	113	96	103	120			,	•	•
		V.L	0, 11	0.615	0.59	0.73	0.43	0.03	0.21	91 0	0.17	0.1	0.15	0.2	0. IS	0.28	0.16	0.48	0.32	0.25	0.16	0.33	9.4	0.51	0.41	6.0	0.3	0.2	0.18	0.18	0.15
		Hď	T.5	7.3	7.5	7.6	7.2	7.17	7.0	6.8	6.7	7.2	6.9	7.1	7.3	7.0	6.8	6.3	8.6	7.2	6.4	7.0	6.9	2.0	7.0	6,7	6.8	7.3	6.3	£.7	7.2
REA	WΜ4	8	550	930	803	480	540	60÷	t 30	403	377	457	\$66	388	1620	D6 6	570	528	582	530	¢6Ü	395	380	960	48 4	£83	(97	514	96)	290	635
ION A		٨L	0,12	0.57	0.64	0.44	0.39	0.25	÷1 ö	9.14	0,15	0.07	0.09	0.15	60.0	0.2	-0.33	0.43	0.34	0.27	0.17	0.43	0.57	0.52	0.48	0.36	0.24	0.18	D. 17	0, 17	0.17
TRACT	~	Ч	7.6		е +		8.1	5.86	6.4	6.6	÷,	6.7	6.7	6.3	5.5	7.0		6.7	6.9	7.1	£.3	8.B	6.7	7.1	7.3	7.1	1.1	7.2	6.B	7.4	7.5
IN EX	WΜ	8	291	1.833	128	161	891	132	158	252	Ę	948	277	163	178	19	173	207	276	295	260	169	300	380	320	296	417	373	213	603	273
JTHEF		Ň		0.27	0.35	0'3	6.9	0.10	0.01	60.03	0.03	-0.16	-0.06	-0.31	-0.16	-0.03	91 TO	0.28	D. 17	0.04	0.07	0.57	0.37	0,37	0.24	0.23	0.08	-0.05	0.38	0.08	0.14
sot	5	Hq	6.9	7.7	8.2 	7.8	7.5	7.85	6.E	7.1	7.1	B.7	6.3	6.9	7.0	6.3	<u>6.</u> 8	6.3	6.2	1.1	. 9	6.5	ψ	8°.8	6.0	6.2	aa Vo	6.E	3.8	9 .0	6.6
	Σ	8	740	633	385	433	716	£77	1162	670	395	690	712	6861	005	170	623	763	720	CB1	790	323	020	206	625	0]9	I	933	1	123	353
		V.L	0.47	Q.79	86.0	0.94	0.99	0.79	0.7	0.57	0.56	110	0.34	0.29	0 39	0.23	<u>0</u> .19	0.45	0.47	0.43	0.31	0.61	0.82	0.98	0.28	0.91	0.76	0.35	0.62	0.57	0.51
Ì	-	На		8	8.6	8.0	7.7	8.12	5.8	6.7	6.7 .	6.4	6.7	6. <u>1</u>	6.5	6.1	. .	6,3	6,5	6,8	6.0	6.3	8.3	8.2	8.5	7.3	1.1	7.2	53	8.8 2.9	4 9
	Σ	8	1540	177	460	830	422	\$C\$	1330	1600	1760	2.94	1631	3 [B	3.66	1530	1221	650	1270	1620	1920	989	2.17	2.3	** 5	1830	3.L7	3.91	Ę	3.6	630
	; ;	Š	=	1.42	1 36	1	2 07	1.5	131	¥6 0	0.92	0.71	0.80	0.97	0.75	08.0	0.51	1.8	0.65	1.05	1.03	÷.	1.58	1, 35	1.57	2,10	1.35		8	: 13	1.28
	Date		04/01/04	0//04/94	04/05/94	03/06/96	10/104	10/08/04	21/09/94	13/10/4	25/10/94	05/12/94	10.01/95	SP/50.PO	28/02/95	09/04/95	03/05/95	30/05/95	05/07/95	36/80/£0	02/00/95	56/01/90	02/11/95	30/11/05	21/12/95	01/02/96	15/03/96	04/04/96	08/05/96	96/90/20	01/07/96

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HELEWATER

GROUNDWATER LEVELS & pH.READINGS 12/1994 - 11/1/96

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	_	Hd	0.5	3.1	3.6	3.9	4.2				
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(E.A	VM4	9	446	\$70	197	184	376				
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		N.L.	1.35	01-1	1.65	1.23	1.28				ater nduc
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GROUNDWMI ER LEVELS - CONDUCTIVITY - pH READINGS 12/1954 to 12/1998

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BAILE Y'S ISLAND	410 WM11 WM12 WM13	D PH MT CO PH MT CO NH MT CO NH MT CO NH	-0.33 560 4.2 015 1840 6.1 0.03 727 6.6 0.06 343 5.9	-0.35 789 4.1 0.28 1636 6.2 -0.13 805 6.2 -0.07 408 6	-0.02 930 3.2 0.2 920 6 0.15 756 6 0.54 372 5.6	0.11 391 4.2 0.3 246 5.8 0.25 544 6.2 0.93 381 5.8	0.13 1320 3.73 0.02 1213 5.9 0.18 660 6.5 0.44 454 6.1	-0.17 556 5.4 0.15 1.92 6.4 0 7.37 6.5 0.22 463 6.2	0.03 490 6.4 0.16 201 6 0.2 2.02 6.9 0.34 586 6.3	0.5 1347 58 058 280 57 0.4 562 62 0.99 359 56	-0 13 9.92 3.8 0.05 1.09 5.9 0.23 645 5.2 0.34 409 41	-0.37 2.6 3.8 -0.14 1153 6.3 0.03 579 6.4 0.37 525 4.5	-0.21 1.9 4.6 005 533 5.8 012 573 5.2 0.6 448 6.2	-0.25 3.3 4.1 -0.15 1.03 6.5 0 555 6.6 0.34 639 6.5	-0.22 1078 5.1 0.21 810 6.6 0.03 5.23 6.7 0.19 206 6.4	-0.22 956 4.9 -0.29 955 6.7 -0.1	-0.33 8 36 5.8 -0.29 1880 7.1 .0.1 855 63 -0.05	0.47 2.56 5.2 0.13 1.4 6 0.1 757 6.5 0.12	0.28 4.51 6.5 0.31 403 6.4 0.4 790 6.5 0.02	0.37 5.6 7 0.41 787 7.7 0.44 804 6.9 0	-0.23 1.61 4.8 0.18 540 6 0.36 611 6.9 0.54 571 6.2	-0.19 940 3.9 0.22 580 6.5 0.32 743 6.4 0.72 941 5.8	-0 23 2.9 4.1 0.1 387 4.4 0.2 625 603 0.67 615 5.6	-0.13 1680 4.6 0.56 430 5.2 1.32 300 5.7 1.6 472 5.4	-0.35 3.17 4.2 0.07 372 5 0.5 305 5.9 1.29 818 5.4	0.67 2.31 5.4 0.06 640 6.5 0.2 617 6.5 0.78 510 6.7	0.15 1990 5.1 -0.13 1156 7.1 0.03 757 6.7 0.54 622 6.5	
BLUE ANGLE PIT	WMB WMB	CD PH WL CD PH WL C	0.15 90 5.8	-0.08 120 5.5	0.6 102 5.7	0.78 90 6	0.48 95 6.2	0.31 101 5.9	0.44 165 6.5	0.35 137 6.3	0.6 125 6.7	0.45 160 5.6	0.62 139 6.2	0.39 148 6.1	0.18 249 6.2	-0.66 507 7.1	-0.11 287 6.2	0.1 230 5.8	0.01 325 6.5	0.34 216 8.1	0.54 105 6.7	0.64 274 3.8	0.61 1082 3.4	1.25 400 5	1 809 5.4	0.6 711 6.3	0.51 661 0.59	
	7MM7	PH W.L CO PH W.L	0.24 161 6.5	0.07 191 6	6.6 0.54 137 5.1	6.1 0.83 158 5.9	6.G 0.56 168 6.5	0.37 162 5.5	0.47 195 6.4	6.9 0.87 186 6.3	7.4 0.74 166 6.1	0.51 131 6.2	7.4 0.7 172 6	0.46 162 6.5	0.39 168 7	0.09 199 7.2	0.01 177 6.6	0.03 225 6.2	0.07 189 6.5	0.19 215 7.4	0.48 164 7	3 91 91 2	0.61 250 6.8	6.8 1.44 209 6	7.1 1.09 202 6.4	7.5 0.59 211 6.5	6.8 0.51 200 E.3	
	WW5 WW6	W.L CD PH W.L CD	0.2	0.18 -0.05	0.43 148 8.6 0.27 80.2	0.53 152 6.3 0.41 77	0.38 163 7.2 0.18 100	0.09	0.32 180 6.9 0.19	0.65 110 7 0.59 94	0.27 128 7.5 0.23 73	0.25 0.1	0.43 109 7.5 0.23 120	0.3		0.28		0.28	0.03	0.33	0.05	0.41 105 7.2 0.23 54	0.36 0.18	1.63 93 6.9 1.03 74	1.13 145 7.4 0.73 78	0.73 236 7 0.43 106	0.51 171 6.7 0.29 106	
THERN EXTRACTION AREA	VM4	CD PH WL CD PH	142 G.9 -0.15 376 7	22 0.8 0.06 384 6.8	94 7.1 035 379 6.9	5C 7 0.53 507 6.7	185 7.6 0.45 677 7.2	20 7.3 0.32 516 7.3	07 5.8 0.34 555 5.9	24 6.7 0.51 440 6.6	33 6.8 0.51 538 6.6	33 6.7 0.49 428 6.8	114 7.2 0.49 440 6.7	48 6.6 0.33 509 6.6	7 12C C2.0 0.9 10	13 7.2 0.11 448 5.8	03 5.8 0.05 909 6.6	28 7.1 -0 03	11 7 3 -0.06	01 7 0.07 1052 6.6	240 6.5 0.32 910 7.2	98 6.8 0.39 790 6.7	316 7.5 0.43 614 72	29 6.5 1.37 480 6.5	191 G.5 1.38 870 G.3	04 5.8 0.9 1051 6.8	216 6.9 0.69 370 6.2	
nos	WW2	PH WL CD PH WL	6.9 0.47 409 6.1 0.02	6.3 0.34 342 5.8 -0.16	6.5 0.53 372 6 0.35	7 0.76 335 5.5 0.67	7 3 0.75 352 6 0.18	6.8 0.64 497 61 0.08	6.4 0.66 357 5.2 0.21 4	6 0.35 409 6 0.79	6 0.93 501 6 0.1	6.2 0.79 440 6.3 0.2	6.2 0.91 478 7 0.41 2	5.8 0.77 304 6 0.13 2	5.7 0.63 347 5.5 -0.01	5.7 0.46 366 6 -0.07 2	5.6 0.33 398 5.8 0.17 2	5.8 0.01 493 6.5 -0.31 2	5.0 0.11 558 7.4 -0.35	5.8 0.21 536 5.8 -0.03	6.1 0.41 488 6.8 0.17	6.5 0.52 466 7.2 0.33	6.7 0.61 478 7.7 0.33	5.8 1.31 523 5 0.41	5.7 1.63 531 5.7 0.63	5.9 1.34 515 5.9 -0.53	6.1 1.21 902 7.1 0.49	
	Date	WL CD	03/12/96 1.28 4.03	06/01/07 1.13 4.2	06/02/97 1.06 2.39	05/03/97 1.41 2.69	11/04/97 1.26 4.17	07/05/97 1.03 4.93	04/06/97 1.13 4.7	E6.E 85.1 7970/10	04/08/97 1.68 4.79	C.C 82.1 70/00/C:	13/10/97 1,93 4.45	10/11/97 1.58 4.34	05/12/07 1.28 4.55	0.61 5	05/02/98 0.47 4.82	71.2 80.0 80/cove.	G3/04/38 0.1 6.47	01/05/98 -0.12 6.27	01/05/98 0.5 1.22	03/07/98 0.9 1.57	03/08/08 1.28 2.65	03/09/98 2.02 2.8	01/10/98 1.88 3.54	11/11/98 1.28 3.59	21/12/98 1.46 2.9	

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GORWATERLEVEL

Piezometer Water Monitoring - Dec.98 too Nov. 00

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	X	<u></u>	54	38	91	74	58	.08	.71	0.6		0.5	39	72			.26	-	.18	.24	.26	0.8	24	.24	.24	-
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s Isla	1-13	1 <u>a</u>	/57	725	522	546	734	669	538	560		533			╞									_		
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ш		3	7.1 0	6.8 0	6.4	6.9 0	6.7 0	6.5 0	6.1 0	5.3		5.11 0	4.9	6.2	-	5.2	5.7	7.3	6.2	5.9	5.6	5	5.6	5.3	6.2	6.1
	M-12	a o	156	442	171	572	535	285	003	459		808	497	608	-	290	285	375	260	220	334	480	625	801	825	495
	ž	C 	0.13 1	0.15 1	0.25	0.05	-0.1	0.47	0.01 1	0.46		00.0	0.15	0.11			0.25	0.15	0.14	-0.1	0.08	0.01	0.02	0.08	70.0	0.07
		4	5.1 -(9 -1	9	6.7 -(5.5	7.1 (5.5 (5.4 (5.4 -(5.8 -(5.2 (4.7	4.9 -(4.2 -(4.7 -(4.4	4.2 (44 (4.6 (4.9 -(5.7 -(5.1 (
	M-11	e e	0661	0063	2600	1285	310	327	302	311		3300	2410	1186		730	775	635	560	538	670	1225	1030	615	810	105
	X	V.L. O	0.15	0.04	0.43	0.45	0.37	0.08	0.18	0.23		0.04	0.13.	0.19		-	0.12	0.23	0.28	0.24	0.05	. 60.0	0.03	0.07	0.01	60.0
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DRAWINGS



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	SURFACE CONTOUR (1m INTERVAL)		GERROA	SAND QUA	RRY				
	APPROXIMATE OUTLINE OF PROPOSED SAND QUARRY APPLICATION		GERROA						
		Client:	CLEARY BRO	OS (BOMBO) PTY	LTD				
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	FOR DETAILS OF SECTIONS C-C' TO G-G' SEE DRAWING 3	Approved	By:	GRW		Date 28	8/02/2005 Drav	wing No.	1



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ADDENDUM TO REPORT on GEOTECHNICAL ASSESSMENT

PROPOSED NORTHERN EXTENSION OF GERROA SAND QUARRY GERROA AND BEACH ROADS, GERROA

Prepared for CLEARY BROS (BOMBO) PTY LTD

Project 37673 September 2006



ADDENDUM TO REPORT on GEOTECHNICAL ASSESSMENT

PROPOSED NORTHERN EXTENSION OF GERROA SAND QUARRY GERROA AND BEACH ROADS, GERROA

Prepared for CLEARY BROS (BOMBO) PTY LTD

Project 37673 September 2006

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GRW:ss Project 37673 28 September 2006

### ADDENDUM TO REPORT ON GEOTECHNICAL ASSESSMENT PROPOSED NORTHERN EXTENSION OF GERROA SAND QUARRY GERROA AND BEACH ROADS, GERROA

#### 1. INTRODUCTION

This addendum to the Douglas Partners Pty Ltd (DP) Report on Geotechnical Assessment, Proposed Northern Extension of Gerroa Sand Quarry, Gerroa and Beach Roads, Gerroa (Project 37673, dated 22 June 2006) presents comments in respect to:

- nutrient levels of the groundwater system in and adjacent to the existing dredge pond
- possible changes of the hydrogeological regime as a result of dredging in the proposed northern extension area.

It is understood that Cleary Bros (Bombo) Pty Ltd (CB) are seeking approval from the Minister for Planning to extract the sand resource, extending some 800 - 900 m northeast of the existing dredge pond area, over a period of about 15 years.

The following comments are based upon a review of the results of:

- groundwater and surface water chemical testing during 2005 and 2006 by Earth2Water Pty Ltd (E2W) and Enviro-Managers Pty Ltd (EM), the results being included in the annual environmental management reports for 2004 and 2005 and a recent analytical report dated 1 September 2006;
- periodic rainfall, extraction face location, dredge pond level and, groundwater monitoring bore data collected by CB;
- groundwater levels in site investigation bores.



Where relevant, information included in the June 2006 DP report is reproduced in the following sections to provide background to the current comments.

#### 2. NUTRIENT LEVELS

As part of the environmental management plan for the operating sand quarry, CB carries out a water monitoring program including sampling and analysis of water samples from the dredge pond on weekly, monthly, six monthly and yearly intervals. The six monthly samples for the dredge pond are tested for a limited number of nutrients; ammonia, nitrate and total phosphorus. During the period January to November 2005, monitoring of groundwater chemistry within the drainage system of Foys Swamp and Blue Angle Creek also included assessment of nutrient levels.

A summary of the available test results for nutrients in the dredge pond during the period 2003 – 2006 is given in Table 1 which also includes ANZECC Guideline values for fresh water environments. The detailed results are included in Appendix A.

Nutrient	Unit	ANZECC						Date					
		Guideline	11/03	05/04	11/04	01/05	06/05	08/05	09/05	10/05	21/05	08/06	08/06
Ammonia as N	mg/L	0.9	<0.02	0.12	0.11	0.063	0.038	<0.01	0.20	0.03	0.19		
Nitrate as N	mg/L	0.7	0.02	<0.02	0.09	0.023	<0.01	0.025	<0.04	0.04	<0.04	0.04	0.04
Nitrite as N	mg/L					<0.01	<0.01	<0.01	0.034	0.022	0.027	0.06	0.05
Total Kjeldahl Nitrogen	mg/L					0.60	0.60	0.40	0.57	0.46	0.39	0.52	0.53
Total Phosphorus as P	mg/L	0.05	0.029	0.04	0.046	0.02	0.02	<0.01	0.014	0.026	<0.01	0.058	0.046
Reactive Phosphorus	mg/L						0.01	<0.01	<0.01	0.016	<0.01		

 Table 1 – Summary of Nutrient Levels in Dredge Pond

The testing results indicate that all samples included in Table 1 were within guideline values with the exception of one Total Phosphorus result from August 2006.



The testing of surface water from the Foy Swamp drainage system and its downstream continuation as Blue Angle Creek indicate that the nutrient levels have at various times exceeded the ANZECC Guideline values for Total Phosphorus. The test values for other nutrients were generally within guideline values.

The maximum values obtained from these sampling locations are compared in Table 2 with maximum values determined from dredge pond samples.

		J		J	
Nutrient	Unit	ANZECC Guideline	Dredge Pond	Blue Angle Creek	Drainage System
Ammonia as N	mg/L	0.9	0.2	0.652	0.25
Nitrate as N	mg/L	0.7	0.09	0.231	0.09
Nitrite as N	mg/L		0.06	0.11	0.114
Total Kjeldahl Nitrogen	mg/L		0.6	2.3	2.4
Total Phosphorus as P	mg/L	0.05	0.058	0.28	1.02
Reactive Phosphorus	mg/L		0.016	0.912	<0.01

Table 2 – Summary of Maximum Nutrient LevelsDredge Pond, Drainage System and Blue Angle Creek

Review of the results in Table 1 indicates that water in the drainage system and Blue Angle Creek has generally higher concentrations of nutrients than the dredge pond.

#### 3. HYDROGEOLOGY

#### 3.1 Hydrogeological Setting

The Gerroa Sand Quarry and the proposed Northern Extension Area are located at the rear of the beach ridge system on low sand dune and sand sheet deposits.

During the DP 2004 investigation, groundwater was noted in the bores and inferred at hole collapse depths at CPT locations at RL 0.5 - 2.5 relative to Australian Height Datum (AHD). In comparison, maximum groundwater levels of approximately RL 0.9, RL 1.9 and RL 1.6 are



indicated by data obtained by Cleary Bros (Bombo) Pty Ltd in water monitoring bores WM 3, WM 4 and WM 5 (all lost at the time of the current investigation), respectively. At the time of the investigation, the dredge pond level was at approximately RL 1.1.

The distribution of measured or inferred groundwater levels during the site investigation is shown on Drawing 1 (see Appendix A). The interpreted distributions of groundwater during earlier monitoring (1996 and 2000) by CB in the Gerroa Sand Quarry and the area extending north-east to adjacent to the Crooked River are shown on Drawings 4 and 5 of the June 2006 DP report. These drawings are included in Appendix A for completeness.

The site investigation data and monitoring indicates:

- the alluvium acts as an aquifer in which there is a moderate variation in groundwater levels but a consistent, north-east trending flow gradient (about 0.3%) adjacent to the dredge pond, possibly reflecting the topographic bedrock high adjacent to southern side of Beach Road.
- elsewhere, there appears to be a generally easterly-trending flow gradient of about 0.1% -0.2% towards the shore.
- there are locally even flatter gradients and reversals of gradient, suggesting that groundwater mounding within the dunes sheds both eastward to the sea and westward to the main drainage canal which continues northward as Blue Angle Creek and thence Crooked River (both of which are tidal).
- the zone of influence of the main canal (at approximately RL 1) appears to extend 80 m to 100 m eastward into the northern-most, 300 m long section of the proposed extension area, where groundwater levels are close to that of the canal.
- there is a relatively rapid change (about 0.8%) in groundwater levels between the area of highest dune (approximately central to the proposed extension area) and the areas to the north (towards the main canal) and to the southwest (towards the dredge pond).



#### 3.2 Rainfall and Pond Level

The site is located immediately adjacent to the Shoalhaven Bight and within an area characterised by a median and mean annual rainfall of about 1165 mm and 1252 mm, respectively (on the basis of over 100 years records at the Kiama and Point Perpendicular monitoring stations). Detailed (daily) rainfall records for the period 1993 to 2000 from the Gerroa Sand Quarry indicate a median and mean annual rainfall of about 1047 mm and 1065 mm, respectively, some 10% to 15% below the anticipated long-term median and average values for the local coastal area and some 6% to 9% greater than the Nowra monitoring station about 13 km inland.

Measurements of the dredge pond level have been compared to available yearly rainfall for the site in the period 1993 – January 2006 and the anticipated median rainfall value (Table 3).

	1993	1994	1995	1996	1997	1998	1999	2000	2005	2006
Maximum Level (AHD)	1.9	1.88	1.875	1.825	1.85	2.175	2.175	1.65	1.65	1.1
Minimum Level (AHD)	1.4	1.27	1.0	1.35	1.35	0.95	1.15	0.95	1.15	1.05
Range of Level (m)	0.5	0.53	0.875	0.475	0.5	1.225	1.025	0.7	0.5	0.05
Total Rainfall (mm)	755	926	1134	866	1101	1440	1309	993	1147	NA
Variation from Median Rainfall (mm)	-410	-239	-31	-299	-64	+275	+144	-172	-18	NA

 Table 3 – Summary of Yearly Rainfall and Dredge Pond Levels

NA = Not available

The results shown in Table 3 indicate that:

- 1998 and 1999 had rainfall in excess of the anticipated median (1165 mm) rainfall.
- the yearly maximum dredge pond level in years of less than median rainfall moved within a limited range (RL 1.65 – 1.9) with an average maximum of RL 1.83.



- the increase in dredge pond level in 1998 and 1999 corresponded closely with the rainfall in excess of the median value.
- the yearly minimum dredge pond level also moved within a limited range (RL 0.95 1.4).
- the minimum dredge pond level (RL 0.95), about the level of the main canal to the north of the site and adjacent to the proposed extension area.
- the maximum dredge pond level (RL 2.175) occurred during the year of highest rainfall (1998) indicating the rapid effect of rainfall on the groundwater regime.

The dredge pond levels have also been compared to daily rainfall events and the results of the 1999 - 2000 record periods (when the dredge pond area was slightly smaller than the current case). The comparison indicates that:

- for daily rainfall events generally in excess of 100 mm or close spaced rainfall events totalling about 100 mm there is a similar rise in the dredge pond level.
- high dredge pond levels declined rapidly towards the minimum (base) level between August 1999 (an above average rainfall period) and June 2000 (within a below average rainfall period) with the decline being stabilised in the February to May 2000 period by some seven rainfall events in the range 10 mm to 50 mm.

When dredge pond levels are compared to the levels in the groundwater monitoring bores, there is an indication that, during years of significantly lower than median rainfall (e.g. 1993), there is a possible localised reversal of groundwater flow at the eastern end of dredge pond (see Drawing 4) compared to a normal or slightly below median rainfall period (e.g. 2000) when flow gradient continues to the northeast (see Drawing 5).

#### 3.3 Rainfall and Evaporation from Dredge Pond

The assessed median rainfall for the Gerroa area is about 1165 mm (in comparison with 1047 mm for the 1993 – 2000 period of recording at the Gerroa Sand Quarry). In comparison, available data for evaporation rates for a NSW south coast area recording station (Ulladulla) indicates that the average daily evaporation rate is of the order of 2.8 mm/day (1022 mm/year), less than either the assessed or measured rainfall data. It is however understood that readings



at the Nowra monitoring station indicate average evaporation of the order of 4 mm/day (about 1460 mm/year). The Ulladulla results are from a coastal location, similar to Gerroa, where temperatures and humidity would be affected by the proximity to the ocean. The Nowra results are from an inland location where temperatures and evaporation would be expected to be higher.

#### 3.4 Effect of Proposed Dredging on Hydrology of the Site

Cleary Bros (Bombo) Pty Ltd are seeking approval from the Minister for Planning to extract the sand resource from the Northern Extension Area, which extends some 800 - 900 m northeast of the existing dredge pond area, over a period of about 15 years. The approximate outline of the area is given on Drawing 1.

It is expected that an excavation face ranging from 80 m to 160 m wide will be progressively moved northward from the current dredge pond and that excavation depths of up to 17 m will potentially be developed to recover sand product. The closest approach of the extraction area to the main canal will be 40 m and there will be a buffer, some 10 - 40 m wide, will be provided for screening bunds and EEC protection between the extraction area and road reserve (Gerroa Road).

It is understood that the Department of Environment and Conservation (DEC) requires information on whether the current and proposed dredging for sand will change the hydrology of the area, specifically in respect to the surrounding Endangered Ecological Communities (EEC). Two EECs are present within the quarry area:

- a Swamp Sclerophyll (Swamp mahogany) Forest on land generally lower than RL 2 on the western side of the dredge pond.
- a strip of Littoral Rainforest on the higher dunes adjacent to Gerroa Road and extending from the northern end of the existing dredge pond to where the bushland starts to thin to the north.

From consideration of the available data, site records and investigation, it is our opinion that:



- the groundwater levels will vary rapidly with rainfall and during periods of substantially above average rainfall could be expected to be similar to those prior to quarrying.
- the base groundwater level is controlled by the main canal system and will be unaltered by the proposed dredging. As a consequence of this (in conjunction with groundwater level rises during substantially above average rainfall), the proposed extraction within the Northern Extension Area sand resource is not expected to result in variation in the range of groundwater levels that have been previously experienced in the current dredge pond or adjacent water monitoring bores.
- the quarrying process could be expected to result in a minimal groundwater gradient between the dredge pond and the main drainage canal to the west (i.e. effectively the same as currently conditions).
- the quarrying process could be expected to result in a groundwater gradient of about 0.8% in the area immediately to the east of the dredge pond (i.e. effectively the same as current conditions). For the base water level of the recorded pond level range, it is expected that the groundwater table will range from approximately RL 1 at the pond to approximately RL 1.4 at the eastern site boundary in the area of the littoral rainforest. It is expected that a similar differential will be present at the time of higher pond levels.
- the available chemical test results for surface and groundwater (see Appendix A) for the period 2003 to 2006 indicate that the water within the current dredge pond satisfied all ANZECC criteria with the exception of the Fe (total) value (which is expected to be naturally elevated in the geological environment including acid sulphate materials and weathering of pyritic iron which forms an accessory mineral of the underlying bedrock of the Berry Formation.
- the proposed extraction within the Northern Extension Area sand resource will not result in variation in the range pH or nutrient levels that have been previously experienced in the current dredge pond (or adjacent water monitoring bores).
- the final water body will not affect the overall commercial use of the aquifer. Average direct
  rainfall to the pond may be excess of the estimated yearly evaporation from the pond
  surface, the pH range (moderately acidic to strongly alkaline) is within the natural range of
  the groundwater and the down gradient landuse is restricted to National Park activities. It is



further understood that the quarrying to date has not resulted in observable changes to the stands of swamp mahogany, the closest of which are located within 5 m of the dredge pond.

 surface water release or groundwater movement from the proposed dredge pond is not expected to result in deterioration (or improvement) of the quality of the water in Blue Angle Creek.

#### DOUGLAS PARTNERS PTY LTD

Reviewed by

**G R Wilson** Principal Dr T J Wiesner Principal

APPENDIX A Notes Relating to this Report Results of Chemical Testing Drawings 1, 4 and 5.



## NOTES RELATING TO THIS REPORT

#### Introduction

These notes have been provided to amplify the geotechnical report in regard to classification methods, specialist field procedures and certain matters relating to the Discussion and Comments section. Not all, of course, are necessarily relevant to all reports.

Geotechnical reports are based on information gained from limited subsurface test boring and sampling, supplemented by knowledge of local geology and experience. For this reason, they must be regarded as interpretive rather than factual documents, limited to some extent by the scope of information on which they rely.

#### **Description and Classification Methods**

The methods of description and classification of soils and rocks used in this report are based on Australian Standard 1726, Geotechnical Site Investigations Code. In general, descriptions cover the following properties strength or density, colour, structure, soil or rock type and inclusions.

Soil types are described according to the predominating particle size, qualified by the grading of other particles present (eg. sandy clay) on the following bases:

Soil Classification	Particle Size
Clay	less than 0.002 mm
Silt	0.002 to 0.06 mm
Sand	0.06 to 2.00 mm
Gravel	2.00 to 60.00 mm

Cohesive soils are classified on the basis of strength either by laboratory testing or engineering examination. The strength terms are defined as follows.

	Undrained
Classification	Shear Strength kPa
Very soft	less than 12
Soft	12—25
Firm	25—50
Stiff	50—100
Very stiff	100—200
Hard	Greater than 200

Non-cohesive soils are classified on the basis of relative density, generally from the results of standard penetration tests (SPT) or Dutch cone penetrometer tests (CPT) as below:

Relative Density	SPT "N" Value (blows/300 mm)	CPT Cone Value (q _c — MPa)
Very loose	less than 5	less than 2
Loose	5—10	2—5
Medium dense	10—30	5—15
Dense	30—50	15—25
Very dense	greater than 50	greater than 25

Rock types are classified by their geological names. Where relevant, further information regarding rock classification is given on the following sheet.

#### Sampling

Sampling is carried out during drilling to allow engineering examination (and laboratory testing where required) of the soil or rock.

Disturbed samples taken during drilling provide information on colour, type, inclusions and, depending upon the degree of disturbance, some information on strength and structure.

Undisturbed samples are taken by pushing a thin-walled sample tube into the soil and withdrawing with a sample of the soil in a relatively undisturbed state. Such samples yield information on structure and strength, and are necessary for laboratory determination of shear strength and compressibility. Undisturbed sampling is generally effective only in cohesive soils.

Details of the type and method of sampling are given in the report.

#### **Drilling Methods.**

The following is a brief summary of drilling methods currently adopted by the Company and some comments on their use and application.

**Test Pits** — these are excavated with a backhoe or a tracked excavator, allowing close examination of the in-situ soils if it is safe to descent into the pit. The depth of penetration is limited to about 3 m for a backhoe and up to 6 m for an excavator. A potential disadvantage is the disturbance caused by the excavation.

Large Diameter Auger (eg. Pengo) — the hole is advanced by a rotating plate or short spiral auger, generally 300 mm or larger in diameter. The cuttings are returned to the surface at intervals (generally of not more than 0.5 m) and are disturbed but usually unchanged in moisture content. Identification of soil strata is generally much more reliable than with continuous spiral flight augers, and is usually supplemented by occasional undisturbed tube sampling.

**Continuous Sample Drilling** — the hole is advanced by pushing a 100 mm diameter socket into the ground and withdrawing it at intervals to extrude the sample. This is the most reliable method of drilling in soils, since moisture content is unchanged and soil structure, strength, etc. is only marginally affected.

**Continuous Spiral Flight Augers** — the hole is advanced using 90—115 mm diameter continuous spiral flight augers which are withdrawn at intervals to allow sampling or in-situ testing. This is a relatively economical means of drilling in clays and in sands above the water



table. Samples are returned to the surface, or may be collected after withdrawal of the auger flights, but they are very disturbed and may be contaminated. Information from the drilling (as distinct from specific sampling by SPTs or undisturbed samples) is of relatively lower reliability, due to remoulding, contamination or softening of samples by ground water.

**Non-core Rotary Drilling** — the hole is advanced by a rotary bit, with water being pumped down the drill rods and returned up the annulus, carrying the drill cuttings. Only major changes in stratification can be determined from the cuttings, together with some information from 'feel' and rate of penetration.

**Rotary Mud Drilling** — similar to rotary drilling, but using drilling mud as a circulating fluid. The mud tends to mask the cuttings and reliable identification is again only possible from separate intact sampling (eg. from SPT).

**Continuous Core Drilling** — a continuous core sample is obtained using a diamond-tipped core barrel, usually 50 mm internal diameter. Provided full core recovery is achieved (which is not always possible in very weak rocks and granular soils), this technique provides a very reliable (but relatively expensive) method of investigation.

#### **Standard Penetration Tests**

Standard penetration tests (abbreviated as SPT) are used mainly in non-cohesive soils, but occasionally also in cohesive soils as a means of determining density or strength and also of obtaining a relatively undisturbed sample. The test procedure is described in Australian Standard 1289, "Methods of Testing Soils for Engineering Purposes" — Test 6.3.1.

The test is carried out in a borehole by driving a 50 mm diameter split sample tube under the impact of a 63 kg hammer with a free fall of 760 mm. It is normal for the tube to be driven in three successive 150 mm increments and the 'N' value is taken as the number of blows for the last 300 mm. In dense sands, very hard clays or weak rock, the full 450 mm penetration may not be practicable and the test is discontinued.

The test results are reported in the following form.

 In the case where full penetration is obtained with successive blow counts for each 150 mm of say 4, 6 and 7

 In the case where the test is discontinued short of full penetration, say after 15 blows for the first 150 mm and 30 blows for the next 40 mm

as 15, 30/40 mm.

The results of the tests can be related empirically to the engineering properties of the soil.

Occasionally, the test method is used to obtain samples in 50 mm diameter thin walled sample tubes in clays. In such circumstances, the test results are shown on the borelogs in brackets.

#### **Cone Penetrometer Testing and Interpretation**

Cone penetrometer testing (sometimes referred to as Dutch cone — abbreviated as CPT) described in this report has been carried out using an electrical friction cone penetrometer. The test is described in Australian Standard 1289, Test 6.4.1.

In the tests, a 35 mm diameter rod with a cone-tipped end is pushed continuously into the soil, the reaction being provided by a specially designed truck or rig which is fitted with an hydraulic ram system. Measurements are made of the end bearing resistance on the cone and the friction resistance on a separate 130 mm long sleeve, immediately behind the cone. Transducers in the tip of the assembly are connected by electrical wires passing through the centre of the push rods to an amplifier and recorder unit mounted on the control truck.

As penetration occurs (at a rate of approximately 20 mm per second) the information is plotted on a computer screen and at the end of the test is stored on the computer for later plotting of the results.

The information provided on the plotted results comprises: —

- Cone resistance the actual end bearing force divided by the cross sectional area of the cone expressed in MPa.
- Sleeve friction the frictional force on the sleeve divided by the surface area expressed in kPa.
- Friction ratio the ratio of sleeve friction to cone resistance, expressed in percent.

There are two scales available for measurement of cone resistance. The lower scale (0-5 MPa) is used in very soft soils where increased sensitivity is required and is shown in the graphs as a dotted line. The main scale (0-50 MPa) is less sensitive and is shown as a full line.

The ratios of the sleeve friction to cone resistance will vary with the type of soil encountered, with higher relative friction in clays than in sands. Friction ratios of 1%—2% are commonly encountered in sands and very soft clays rising to 4%—10% in stiff clays.

In sands, the relationship between cone resistance and SPT value is commonly in the range:—

 $q_c$  (MPa) = (0.4 to 0.6) N (blows per 300 mm)

In clays, the relationship between undrained shear strength and cone resistance is commonly in the range:—

$$q_c = (12 \text{ to } 18) c_u$$

Interpretation of CPT values can also be made to allow estimation of modulus or compressibility values to allow calculation of foundation settlements.

Inferred stratification as shown on the attached reports is assessed from the cone and friction traces and from experience and information from nearby boreholes, etc. This information is presented for general guidance, but must be regarded as being to some extent interpretive. The test method provides a continuous profile of engineering properties, and where precise information on soil classification is required, direct drilling and sampling may be preferable.



#### **Hand Penetrometers**

Hand penetrometer tests are carried out by driving a rod into the ground with a falling weight hammer and measuring the blows for successive 150 mm increments of penetration. Normally, there is a depth limitation of 1.2 m but this may be extended in certain conditions by the use of extension rods.

Two relatively similar tests are used.

- Perth sand penetrometer a 16 mm diameter flatended rod is driven with a 9 kg hammer, dropping 600 mm (AS 1289, Test 6.3.3). This test was developed for testing the density of sands (originating in Perth) and is mainly used in granular soils and filling.
- Cone penetrometer (sometimes known as the Scala Penetrometer) — a 16 mm rod with a 20 mm diameter cone end is driven with a 9 kg hammer dropping 510 mm (AS 1289, Test 6.3.2). The test was developed initially for pavement subgrade investigations, and published correlations of the test results with California bearing ratio have been published by various Road Authorities.

#### Laboratory Testing

Laboratory testing is carried out in accordance with Australian Standard 1289 "Methods of Testing Soil for Engineering Purposes". Details of the test procedure used are given on the individual report forms.

#### **Bore Logs**

The bore logs presented herein are an engineering and/or geological interpretation of the subsurface conditions, and their reliability will depend to some extent on frequency of sampling and the method of drilling. Ideally, continuous undisturbed sampling or core drilling will provide the most reliable assessment, but this is not always practicable, or possible to justify on economic grounds. In any case, the boreholes represent only a very small sample of the total subsurface profile.

Interpretation of the information and its application to design and construction should therefore take into account the spacing of boreholes, the frequency of sampling and the possibility of other than 'straight line' variations between the boreholes.

#### **Ground Water**

Where ground water levels are measured in boreholes, there are several potential problems;

- In low permeability soils, ground water although present, may enter the hole slowly or perhaps not at all during the time it is left open.
- A localised perched water table may lead to an erroneous indication of the true water table.
- Water table levels will vary from time to time with seasons or recent weather changes. They may not be

the same at the time of construction as are indicated in the report.

• The use of water or mud as a drilling fluid will mask any ground water inflow. Water has to be blown out of the hole and drilling mud must first be washed out of the hole if water observations are to be made.

More reliable measurements can be made by installing standpipes which are read at intervals over several days, or perhaps weeks for low permeability soils. Piezometers, sealed in a particular stratum, may be advisable in low permeability soils or where there may be interference from a perched water table.

#### **Engineering Reports**

Engineering reports are prepared by qualified personnel and are based on the information obtained and on current engineering standards of interpretation and analysis. Where the report has been prepared for a specific design proposal (eg. a three storey building), the information and interpretation may not be relevant if the design proposal is changed (eg. to a twenty storey building). If this happens, the Company will be pleased to review the report and the sufficiency of the investigation work.

Every care is taken with the report as it relates to interpretation of subsurface condition, discussion of geotechnical aspects and recommendations or suggestions for design and construction. However, the Company cannot always anticipate or assume responsibility for:

- unexpected variations in ground conditions the potential for this will depend partly on bore spacing and sampling frequency
- changes in policy or interpretation of policy by statutory authorities
- the actions of contractors responding to commercial pressures.

If these occur, the Company will be pleased to assist with investigation or advice to resolve the matter.

#### **Site Anomalies**

In the event that conditions encountered on site during construction appear to vary from those which were expected from the information contained in the report, the Company requests that it immediately be notified. Most problems are much more readily resolved when conditions are exposed than at some later stage, well after the event.

#### Reproduction of Information for Contractual Purposes

Attention is drawn to the document "Guidelines for the Provision of Geotechnical Information in Tender Documents", published by the Institution of Engineers, Australia. Where information obtained from this investigation is provided for tendering purposes, it is recommended that all information, including the written report and discussion, be made available. In circumstances where the discussion or comments section



is not relevant to the contractual situation, it may be appropriate to prepare a specially edited document. The Company would be pleased to assist in this regard and/or to make additional report copies available for contract purposes at a nominal charge.

#### **Site Inspection**

The Company will always be pleased to provide engineering inspection services for geotechnical aspects of work to which this report is related. This could range from a site visit to confirm that conditions exposed are as expected, to full time engineering presence on site.

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D	Parameter	Units	11/19/2003	05/27/2004	11/26/2004
South Reed Bed	Ammonia	mg/i	<0.02	0.29	<0.02
	Chloride	mg/l	66	65	94
· .	Conductivity	uS/cm@25C	-	620	•
	Nitrate	mg/l	<0.02	<0.02	0.08
	Oil & Grease	mg/l	<0.1	<0.1	0.2
	рН	pH units	8	7.3	6.5
	Sulphate	mg/l	73	82	8
	Total Phosphorus	mg/l	0.057	0.07	0.068
Existing Works Area	Ammonia	mg/l	<0.02	0.12	0.11
	Chloride	mg/l	64	68	66
	Conductivity	uS/cm@25C	557	620	530
	Nitrate	mg/l	0.02	<0.02	0.09
• •	Oil & Grease	mg/l	<0.1	3.2	<0.1
	<u>pH</u>	pH units	8.2	7.3	6.8
· · •	Sulphate	mg/l	70	86	88
	Lotal Phosphorus	mg/l	0.029	0.04	0.046

All-data provided by Cleary Bros (Bombo) Pty Ltd -

Analysis performed by Enviro-Managers Pty Ltd

Analytical Report - Enviro-Man	agers
Client:	Cleary Bros (Bombo) Pty Ltd
	Springhill Rd
	CONISTON
	NSW, 2500
Phone:	0408 226 190
Fax:	02 4276 1168
Contact Name:	Mr Ron Bryant
Client Reference:	Gerroa

Fax:	02 4276 1168				
Contact Name:	Mr Ron Bryant				
Client Reference:	Gerroa				
-	Report Number:	W05/2090	W05/2090	W05/4409	W05/4409
Results:	Sample Received:	26/05/2005	26/05/2005	21/11/2005	21/11/2005
Client Id		Ex Works	Sth Reed	Ex Works	Sth Reed
Laboratory Id		W12803/001	W12803/002	W15172/001	W15172/002
Ammonia (NH3 as N)					
Method: WCM084	Units:mg/L	0.04	0.03	0.61	0.38
Chloride					0
Method: WCM055	Units:mg/L	72	56	80	59
Nitrate (NO3 as N)				3	8
Method: WCM085	Units:mg/L	0.06	0.03	<0.04	0.09
Oil & grease					
Method:APHA 5520	Units:mg/L	<0.1	60.1	<0.1	<0.1
рН					
Method: APHA 4500 H B	Units:pH units	6.8	6.5	77	68
Sulphate (SO4)					0
Method: APHA 3120 B	Units:mg/L	123	8.6	138	16
Total Phosphorus (TP)					2
Method: WCM090	Units:mg/L	<0.002	<0.002	<0.005	<0.005
Method(s):					
APHA 3120 B	Sulphate (SO4)				
APHA 4500 H B	Hd				
APHA 5520	Oil & grease				
WCM055	Chloride				
WCM084	Ammonia (NH3 as N)				
WCM085	Nitrate (NO3 as N)				
WCM090	Total Phosphorus (TP)				

	Report Number:		W05/2090	W05/4409		W05/2040	WINSIAANG	
Results:	Sample Received:	Guidelines	26/05/2005	21/11/2005	Average	26/05/2005	21/11/2005	Averade
		ANZECC (2000)						202
Client Id		Freshwater	Ex Works	Ex Works		Sth Reed	Sth Reed	
Laboratory Id		-	W12803/001	W15172/001		W12803/002	W15172/002	
Ammonia (NH3 as N)					3			
Method: WCM084	Units:mg/L	0.9	0.04	0.61	0 325	0.03	0.38	0 205
Chloride						2222	0000	222
Method: WCM055	Units:mg/L	250	72	80	76	56	20	57 G
Nitrate (NO3 as N)					2	3	3	2.12
Method: WCM085	Units:mg/L	0.7	0.06	<0.04		0.03	60 U	0.06
Oil & grease								5
Method:APHA 5520	Units:mg/L	No Visible Sheen	<0.1	01		<0.1	<0.1	
pH			İ					
Method:APHA 4500 H B	Units:pH units	6 - 9.0	6.8	7.7	7.25	65	89	6.65
Sulphate (SO4)								2
Method:APHA 3120 B	Units:mg/L	250	123	138	130.5	86	16	10 2
Total Phosphorus (TP)				- -		1	2	0.41
Method:WCM090	Units:mg/L	AN	<0.002	<0.005	1	<0.002	<0.005	

Table 1A- Gerroa 6 Monthly Surface Water Results (Ex Works, Sth Reed)

Gerroa monitoring data final DP 19-1-06.xls

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# **TABLE 1:** Surface Water Analytical Results Foys Swamp, Blue Angle Creek and Gerroa Sand Quarry

Toys Swallip, Dide Angle Cleek and			у	1			1			r	r	1	r	1	T	r	r	1	1	1	r
				Sample ID	M.DRAIN-1	M.DRAIN-1	M.DRAIN-1	M.DRAIN-1	M.DRAIN-1	M.DRAIN-1	M.DRAIN-1	M.DRAIN-2	M.DRAIN-2	M.DRAIN-2	M.DRAIN-2	M.DRAIN-2	M.DRAIN-2	M.DRAIN-2	BA Creek	BA Creek	BA Creek
					Main Drain-	Main Drain-	Main Drain-	Main Drain-	Main Drain-	Main Drain-	Main Drain-	Main Drain-	Main Drain-	Main Drain-	Main Drain- dn	Main Drain- dn	Main Drain- dn	Main Drain- dn	Blue Angle	Blue Anale	Blue Angle
Cleary Bros (Bombo) Pty Ltd	Water Mor	nitoring Pro	gram	Sample ID	up stream	up stream	up stream	up stream	up stream	up stream	up stream	dn stream	dn stream	dn stream	stream	stream	stream	stream	Creek	Creek	Creek
		r	1			•	•	•	•		•								-		
	Units	LOR	ANZECC 20	00 Guidelines	Wet Weather	Dry Weather	Dry Weather	Dry Weather	Dry Weather	Dry Weather	Dry Weather	Wet Weather	Dry Weather	Dry Weather	Dry Weather	Dry Weather	Dry Weather	Dry Weather	Wet Weather	Dry Weather	Dry Weather
Matala (tatal)			Marine	Fresh	07/04/05	00/04/05	0/00/05	0/00/05	04/00/05	04/40/00	04/44/05	07/04/05	00/04/05	0/00/05	0/00/05	04/00/05	04/40/00	04/44/05	0/00/05	00/04/05	0/00/05
	m a/l	0.0001	ID	0.055	27/01/05	28/04/05	2/06/05	3/08/05	21/09/05	21/10/06	21/11/05	27/01/05	28/04/05	2/06/05	3/08/05	21/09/05	21/10/06	21/11/05	3/02/05	28/04/05	2/06/05
	nig/L	0.0001	ID	0.055	0.004		0.99	2.1			0.20	0.000		0.001	0.004			0.12	0.000		0.59
Arsenic	mg/L	0.001	ID	0.013	<0.001		<0.001	<0.001	-	-	<0.001	0.002		<0.001	<0.001	-	-	<0.001	0.002		0.004
Beryllium	mg/L	0.001	ID	ID	<0.001							<0.001							0.001		
Barium	mg/L	0.001	ID	ID	0.016							0.015							0.014		
Cadmium	mg/L	0.0001	0.0007	0.0002	0.0002		<0.0001	<0.0001			<0.001	0.0003		<0.0001	<0.0001			<0.001	0.0004		<0.0001
Chromium (Total)	mg/L	0.001	0.0274	ID	0.003		<0.001	0.001				0.003		<0.001	<0.001				0.003		<0.001
Cobalt	mg/L	0.001	0.001	ID	0.01							0.009							0.004		
Copper	mg/L	0.001	0.0013	0.0014	<0.001		0.003	0.006			<0.001	<0.001		0.003	0.002			<0.001	0.01		0.006
Lead	mg/L	0.001	0.0044	0.0034	<0.001		<0.001	<0.001			< 0.001	<0.001		<0.001	<0.001			<0.001	0.01		<0.001
Zinc	mg/L	0.005	0.015	0.008	0.013		0.012	0.031			< 0.005	0.012		0.023	0.009			< 0.005	0.023		0.022
Manganese	mg/L	0.001	ID	1.9	0.596							0.543							0.14		
Nickel	mg/L	0.001	0.007	0.011	0.01							0.009							0.006		
Vanadium	ma/L	0.01	0.1	ID	< 0.01							< 0.01							< 0.01		
Total Iron	ma/L	0.005	ID	ID	0.75		0.71	2.48			0.30	0.42		0.49	0.52			0.11	<0.1		2.07
Mercury	mg/l	0.0001	0.0001	0,00006	<0.0001		<0.0001	<0.0001			<0.0005	<0.0001		<0.0001	<0.0001			<0.0005	0.0001		<0.0001
		0.000.	0.0001	0.00000	1010001			1010001							1010001			1010000	0.0001		
Weak Acid Dissociable Cvanide	ma/l	0.005	0.004	0.007								<0.0050									
Treak Acia Dissectable Oyunae	iiig/E	0.000	0.004	0.007								<0.0000									
Nutrients				1								1							1		
Fluoride	ma/l	01		1															0.1		
Ammonia as N	mg/L	0.01	0.91	0.9	0.052	0.028	0.073	0.084	0.18	<0.02	0.25	0.096	0.031	0.047	0.02	0.17	0.02	0.19	0.048	0 282	0.53
Nitrato as N	mg/L	0.01	0.91	0.3	<0.002	<0.020	0.076	0.004	<0.10	0.02	<0.20	<0.000	<0.001		0.02	<0.04	0.02	<0.01	<0.040	0.202	~0.010
Nitrito as N	mg/L	0.01		0.7	<0.010	<0.010	~0.020	0.014	<0.04	0.03	0.04	<0.010	<0.010	<0.010	<0.023	<0.04	0.03	0.005	0.014	<0.002	0.026
Total Kieldehl Nitregen es N	mg/L	0.01			0.010	0.60	2.40	0.013	<0.002	0.014	0.004	0.010	<0.010	0.010	<0.010	<0.002	0.014	0.003	1.90	1.00	0.020
Total Recentering on D	mg/L	0.10		0.05	0.80	0.00	2.40	2.20	0.04	0.55	0.03	0.70	0.30	0.90	0.00	0.80	0.40	0.47	1.60	0.00	2.30
Reactive Bhasekerry	mg/L	0.01		0.05	0.04	0.01	0.25	1.02	<0.005	<0.005	< 0.005	0.01	<0.010	0.02	<0.01	<0.005	<0.005	<0.005	0.10	0.06	0.20
Reactive Phosphorus	mg/L	0.01			<0.010	<0.01	<0.010	<0.010	<0.004	<0.004	<0.004	0.912	<0.010	<0.010	<0.010	<0.004	<0.004	<0.004		<0.010	<0.010
		0.04				0.47	0.70	0.00	7.00	7.00	0.00		0.00	0.74	0.00	7.00	7.00	0.00		0.00	0.07
PH (lab)	pH Unit	0.01				6.17	6.72	6.33	7.20	7.00	6.80		6.90	6.71	6.68	7.00	7.00	6.90		6.80	6.87
Total Dissolved Solids (TDS)	mg/L	1			552	2460	646	1230	4515.8	5808.9	3577.8		1240	644	1730	3912.8	5453.8	4127.2	303	2330	18500
Electrical Conductivity	uS/cm	1				_			6740	8670	5340			-		5840	8140	6160			
Suspended Solids (SS)	mg/L	1			11	7		3				6	10	-	9				23	13	
Total Hardness	mg/L	1			164														73		
Major lons							07				50								40		005
	mg/L	1		l	28		37			-	53						-	63	13		225
Magnesium	mg/L	1			20		26				93							110	10		587
Sodium	mg/L	1			87		134				669			-				789	39		4460
Potassium	mg/L	1			10		10				31			-				36	6		176
Bicarbonate as CaCO3	mg/L	1		ļ	2		31												10		90
Total Alkalinity	mg/L	1			2		31												10		90
Sulphate as SO4	mg/L	1			159		182				293							392	63		1200
Chloride	mg/L	1			130		212				1308							1684	64.5		8930
SAR																					
Calcium + Magnesium (meq/L)					3.04		3.99				10.29							12.19	1.47		59.51
Sodium (meq/L)					3.78		5.83				29.10							34.32	1.70		194.01
SAR= Na / Sqrt (Ca+ Mg) / 2)					1.23		1.41				2.27							2.47	0.86		5.46
SAR- Sodium Absorption Ratio					3.07		4.13				12.83							13.90	1.98		35.57
SAR Hazard Ranking					Low		Low				Med							Med	Low		V High

Note:

SAR Hazard ranking based on Fetter, 1994. Low = 2 to 10, Med = 7 to 18, High= 11 to 26, V High= 26+ nr - no recommended NSW guidelines NA - Not Available TDS= EC*0.67 (approximate) calculation in italics (Data from Sept to Dec05)

3 Exceeds ANZECC 2000 trigger values (marine and/or fresh water)

3

# **TABLE 1:** Surface Water Analytical ResultsFoys Swamp, Blue Angle Creek and Gerroa Sand Quarry

Toys on amp, Blac Angle of certain	001104 00		<b>y</b>								
				Sample ID	BA Creek	BA Creek	BA Creek	BA Creek	SW Drain	SW Drain	SW Drain
Cleary Bros (Bombo) Pty Ltd	Water Moi	nitoring Pro	ogram	Sample ID	Blue Angle Creek	Blue Angle Creek	Blue Angle Creek	Blue Angle Creek	SW Drain	SW Drain	SW Drain
	Units	LOR	ANZECC 20	00 Guidelines	Dry Weather	Dry Weather	Dry Weather	Dry Weather	Wet Weather	Dry Weather	Dry Weathe
Motols (total)			Marine	Fresh	2/02/05	21/00/05	21/10/06	24/44/05	27/04/05	2/06/05	24/44/2005
Metals (total)	ma/l	0.0001	ID	0.055	3/08/05	21/09/05	21/10/06	21/11/05	27/01/05	2/06/05	21/11/2005
Areonic	mg/L	0.0001		0.055	0.003			<0.02	0.004		
Beryllium	mg/L	0.001		0.013	0.003			<0.001	<0.004		
Barium	mg/L	0.001	ID								
Cadmium	mg/L	0.001	0.0007	0.0002	<0.0001			<0.001	<0.013		
Chromium (Total)	mg/L	0.0001	0.0007	0.0002	0.001			<0.001	0.003		
Cobalt	mg/L	0.001	0.0214		0.001				0.003		
Copper	mg/L	0.001	0.0013	0.001/	0.008			<0.001	0.002		
Lead	mg/L	0.001	0.0013	0.0014	<0.000			<0.001	<0.002		
Zinc	mg/L	0.001	0.0044	0.0034	0.016			<0.001	0.412		
Manganese	mg/L	0.000	0.015	1.0	0.010			<0.000	0.003		
Nickel	ma/l	0.001	0.007	0.011			+	<u> </u>	<0.00		
Vanadium	ma/l	0.01	0.007				+	<u> </u>	<0.005		
Total Iron	mg/L	0.005	ID		7 12			0.28	2 16		
Mercury	mg/L	0.0001	0.0001	0,000.6	<0.0001			<0.0005	<0.0001		
	iiig/L	0.0001	0.0001	0.00000	20.0001			<0.0000	<b>CO.0001</b>		
Weak Acid Dissociable Cyanide	mg/L	0.005	0.004	0.007							
Nutrients											
Fluoride	mg/L	0.1									
Ammonia as N	mg/L	0.01	0.91	0.9	0.652	0.22	0.12	0.26	0.074	0.096	0.15
Nitrate as N	mg/L	0.01	ID	0.7	0.231	0.05	0.08	< 0.04	<0.010	0.016	< 0.04
Nitrite as N	mg/L	0.01			0.114	0.005	0.021	0.004	0.012	<0.010	0.11
Total Kjeldahl Nitrogen as N	mg/L	0.10			0.90	0.73	0.58	0.44		1.50	1.5
Total Phosphorus as P	mg/L	0.01		0.05	0.05	< 0.005	<0.005	< 0.005	0.31	0.13	0.05
Reactive Phosphorus	mg/L	0.01			<0.010	<0.004	<0.004	<0.004		0.123	0.043
PH (lab)	nH I Init	0.01			6.89	7.00	6.90	6 70		6.87	75
Total Dissolved Solids (TDS)	ma/l	1			3620	3752	7624.6	4107.1	203	243	520
Electrical Conductivity	uS/cm	1			0020	5600	11380	6130	200	245	520
Suspended Solids (SS)	ma/l	1			26	0000	11000	0100	20		
Total Hardness	mg/L	1			20				52		
Major lons											
Calcium	mg/L	1						63	10		
Magnesium	mg/L	1						111	7		
Sodium	mg/L	1						808	26		
Potassium	mg/L	1						37	7		
Bicarbonate as CaCO3	mg/L	1							39		
Total Alkalinity	mg/L	1							39		
Sulphate as SO4	mg/L	1						345	8		
Chloride	mg/L	1						1635	49		
SAR											
Calcium + Magnesium (meq/L)								12.27	1.07		
Sodium (meq/L)								35.15	1.13		
SAR= Na / Sqrt (Ca+ Mg) / 2)			<u> </u>				ļ	2.48	0.73		
SAR- Sodium Absorption Ratio			ļ	ļ	ļ		ļ	14.19	1.54		
SAR Hazard Ranking								Med	Low		
Note:											

SAR Hazard ranking based on Fetter, 1994. Low = 2 to 10, Med = 7 to 18, High= 11 to 26, V High= 26+ nr - no recommended NSW guidelines NA - Not Available TDS= EC*0.67 (approximate) calculation in italics (Data from Sept to Dec05)



## **TABLE 1:** Surface Water Analytical Results Foys Swamp, Blue Angle Creek and Gerroa Sand Quarry

Foys Swamp, Dive Angle Creek and	Gentua Sa	anu Quan	у							1	1						1	·	
				Sample ID	W Drain	W Drain	W Drain	NW Drain	NW Drain	NW Drain	NW Drain	NW Drain	NW Drain	Dredge Pond	Dredge Pond	Dredge Pond	Dredge Pond	Dredge Pond	Dredge Pond
Cleary Bros (Bombo) Pty Ltd	Water Mo	nitoring Pro	ogram	Sample ID	W Drain	W Drain	W Drain	NW Drain	NW Drain	NW Drain	NW Drain	NW Drain	NW Drain	MD Pond	MD Pond	MD Pond	MD Pond	MD Pond	MD Pond
	Units	LOR	ANZECC 200	00 Guidelines	Wet Weather	Dry Weather	Dry Weather	Wet Weather	Dry Weather	Dry Weather	Dry Weather	Dry Weather	Dry Weather	Wet Weather	Dry Weather	Dry Weather	Dry Weather	Dry Weather	Dry Weather
			Marina	Frosh														()	
Metals (total)			Warne	Tiesii	27/01/2005	2/06/2005	21/11/2005	27/01/2005	2/06/05	3/08/05	21/09/05	21/10/05	21/11/05	27/01/05	2/06/05	3/08/05	21/09/05	21/10/05	21/11/05
Aluminium (PH>6.5,)	mg/L	0.0001	ID	0.055					3.04	2.83			18		1.42	1.28		ļļ	1.9
Arsenic	mg/L	0.001	ID	0.013	0.003			<0.001	<0.001	<0.001			<0.001	0.004	0.003	0.002		ļļ	<0.001
Beryllium	mg/L	0.001	ID	ID	<0.001			0.002						<0.001				ļ]	<b> </b>
Barium	mg/L	0.001	ID	ID	0.03			0.028						0.01				ļ]	<b> </b>
Cadmium	mg/L	0.0001	0.0007	0.0002	0.0005			0.0003	<0.0001	<0.0001			<0.001	0.0002	<0.0001	<0.0001		ļ]	<0.001
Chromium (Total)	mg/L	0.001	0.0274	ID	0.003			0.002	<0.001	<0.001				0.004	0.002	0.001		ļ]	<b> </b>
Cobalt	mg/L	0.001	0.001	ID	<0.001			0.038						<0.001				<u>ا</u> ـــــــــــا	ļ
Copper	mg/L	0.001	0.0013	0.0014	0.025			0.003	0.002	0.004			0.002	<0.001	0.002	0.003		J	<0.001
Lead	mg/L	0.001	0.0044	0.0034	0.001			<0.001	<0.001	<0.001			<0.001	<0.001	<0.001	<0.001		<u>ا</u> ـــــــــــا	<0.001
Zinc	mg/L	0.005	0.015	0.008	0.072			1.72	0.035	0.019			0.053	0.015	0.05	0.012		J	< 0.005
Manganese	mg/L	0.001	ID	1.9	0.004			0.028						0.003					<u> </u>
Nickel	mg/L	0.001	0.007	0.011	<0.01			<0.01						<0.01					<u> </u>
Vanadium	mg/L	0.01	0.1	ID	0.022			0.045						<0.005					<u> </u>
Total Iron	mg/L	0.005	ID	ID	0.83			8.64	0.39	1.4			0.58	0.77	1.14	0.78			0.57
Mercury	mg/L	0.0001	0.0001	0.00006	<0.0001			0.0016	<0.0001	<0.0001			<0.0005	<0.0001	<0.0001	<0.0001		i J	<0.0005
																		l l	
Weak Acid Dissociable Cyanide	mg/L	0.005	0.004	0.007										<0.0050				[ ]	
																		[]	
Nutrients																			
Fluoride	mg/L	0.1												0.2					
Ammonia as N	mg/L	0.01	0.91	0.9	0.055	0.043	0.35	0.066	0.046	0.059	0.23	<0.02	0.31	0.063	0.038	<0.010	0.20	0.03	0.19
Nitrate as N	mg/L	0.01	ID	0.7	0.011	<0.010	< 0.04	<0.010	0.301	<0.010	<0.04	<0.04	<0.04	0.023	<0.010	0.025	< 0.04	0.04	< 0.04
Nitrite as N	mg/L	0.01			<0.010	<0.010	0.022	<0.010	<0.010	<0.010	<0.002	0.014	<0.002	<0.010	<0.010	<0.010	0.034	0.022	0.027
Total Kjeldahl Nitrogen as N	mg/L	0.10				1.00	5.7		1.20	2.10	0.62	0.50	0.46	0.60	0.60	0.40	0.57	0.46	0.39
Total Phosphorus as P	mg/L	0.01		0.05	0.18	0.12	0.4	0.08	0.03	0.35	0.006	< 0.005	<0.005	0.02	0.02	<0.01	0.014	0.026	< 0.005
Reactive Phosphorus	mg/L	0.01				0.066	< 0.004		<0.010	<0.010	< 0.004	< 0.004	< 0.004		0.01	<0.010	< 0.004	0.016	< 0.004
PH (lab)	pH Unit	0.01				7.43	8.20		4.43	5.06	6.40	6.40	3.80		7.06	7.47	7.60	7.40	7.50
Total Dissolved Solids (TDS)	mg/L	1			306	220	720	914	406	410	1742	2639.8	1815.7	360	324	336	406.69	425.45	589.6
Electrical Conductivity	uS/cm	1									2600	3940	2710				607	635	880
Suspended Solids (SS)	mg/L	1			21			31		50				11		7			
Total Hardness	mg/L	1			103			212										[]	
																		(	
Major lons																			
Calcium	mg/L	1			24			32	18				60	43	42				47
Magnesium	mg/L	1			10			32	17				65	12	11				14
Sodium	mg/L	1			41			123	78				257	46	42			J	51
Potassium	mg/L	1			7			10	4				12	4	4				5.2
Bicarbonate as CaCO3	mg/L	1			75			<1	<1					48	47				
Total Alkalinity	mg/L	1			75			<1	<1					48	47				
Sulphate as SO4	mg/L	1			32			264	140				533	109	104				134
Chloride	mg/L	1			61.1			166	110				445	65.5	71				82
SAR																			
Calcium + Magnesium (meq/L)					2.02			4.23	2.30				8.34	3.13	3.00				3.50
Sodium (meq/L)					1.78			5.35	3.39				11.18	2.00	1.83				2.22
SAR= Na / Sqrt (Ca+ Mg) / 2)					1.01			1.45	1.07				2.04	1.25	1.22				1.32
SAR- Sodium Absorption Ratio					1.77			3.68	3.17				5.47	1.60	1.49				1.68
SAR Hazard Ranking					Low			Low	Low				Low	Low	Low				Low

Note:

SAR Hazard ranking based on Fetter, 1994. Low = 2 to 10, Med = 7 to 18, High= 11 to 26, V High= 26+ nr - no recommended NSW guidelines NA - Not Available TDS= EC*0.67 (approximate) calculation in italics (Data from Sept to Dec05)



Enviro-Managers Pty Ltd bading as Ecowher Environmental ABN 18 072 428 810 www.ecowise.com.eu (Subsidiary of ActewAGL)

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#### Analytical Report

## Report No: W06/3224

Results						
Client Id		Gerroa East	West			
Laboratory Id		W18849/001	W18849/002			<u> </u>
Nitrate (NO3 as N) Method: WCM085	Unils: mg/L	0.04	0.04		n an an Anna an Anna an Anna an Anna Anna an Anna an Anna an Anna an Anna Anna an Anna an Anna an Anna an Anna an Anna an Anna an Anna an Anna an Anna	
NOx (as N) Method: WCM085	Units: mg/L	0.06	0.05			<u>[]</u>
Phosphate as P Method: WCM089	Units: mg/L	<0.004	€0,004			
Total Nitrogen (TN): Method: WCM083	Units: mg/L	0.52	0.53	e d'é i Vyye <del>s</del>		
Total Phosphorus (TP) Method: WCM090	Units: mg/L	0.056	0.046			

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WCM083	Total Nitrogen (TN)
WCM085	Nitrate (NO3 as N)
WCM085	NOx (as N)
WCM089	Phosphate as P
WCM090	Total Phosphorus (TP)

Date Reported: Friday September 1, 2006

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Page 2 of 2



LEGEND
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- Ð BORE (DOUGLAS PARTNERS)
- SURFACE CONTOUR (1m INTERVAL)
- APPROXIMATE OUTLINE OF PROPOSED SAND QUARRY APPLICATION _
  - GROUNDWATER MONITORING BORE (APPROX. LOCATION ONLY)
- **1**.5 GROUNDWATER LEVEL (AHD) EITHER ESTIMATED FROM FALL IN DEPTH IN CPTS OR DIRECT MEASUREMENT OF GROUNDWATER IN BORES 1.5

MAXIMUM GROUNDWATER LEVEL FROM MONITORING



lbourne,Perth, Wollongong Campbelltown, Cairns, Darwin Townsville

1 Addendum

Title	GROUND PROPOSI GERROA GERROA	WATER LEV ED NORTHE SAND QUAF	ELS DE RN EXT RRY	CEMBER 2004 ENSION			
Client:	CLEARY BRO	S (BOMBO) PTY I	LTD				
Drawn By:	GRW	Scale:	As shown	Project No.	37673B	Office:	Sydney
Approved	Bv:	GRW					

Date

5/09/2006 Drawing No.



